

**GDI ENGINEERING**



**Weber Residence**

Residential

**Wimberley, Texas**

**GENERAL NOTES**

1. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE PROVISIONS OF THE 2018 INTERNATIONAL BUILDING CODE.
2. THE BUILDING STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING CODE
3. ALL DIMENSIONS AND CONDITIONS OF EXISTING CONSTRUCTION SHALL BE VERIFIED AT THE JOB SITE PRIOR TO THE PREPARATION OF SHOP DRAWINGS. DIFFERENCES BETWEEN EXISTING CONSTRUCTION AND THAT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE REFERRED TO THE ARCHITECT. DIFFERENCES SHALL ALSO BE CLOUDED ON THE SHOP DRAWINGS. CUTTING OR CORING OF ANY STRUCTURAL CONCRETE OR STEEL ELEMENTS SHALL BE COORDINATED WITH THE ENGINEER.
4. ITS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER

**PRESCRIBED LOADS:**

**CONSTANT LOADS:**

- . SELF-WEIGHT
- . DEAD LOAD OF THE ROOF STRUCTURE, SECOND FLOOR AND FOUNDATION SLAB. IMPOSED LOADS:
- . WIND LOAD (acc. ASCE 7-22)
- . ROOF LIVE LOAD (acc. ASCE 7-22)
- . LIVE LOAD ON SECOND FLOOR AND FOUNDATION SLAB (acc. ASCE 7-22)

5. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.
6. THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED FOR DETERMINATION OF QUANTITY, LENGTH OR FIT OF MATERIALS.
7. CONTRACTOR SHALL COMPARE STRUCTURAL AND ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.
8. CONTRACTOR SHALL ENSURE THAT CONSTRUCTION MATERIALS WHOSE WEIGHT EXCEEDS THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS ARE NOT STORED ON STRUCTURALLY SUPPORTED FLOOR OR ROOF FRAMING.
9. REPRODUCTION OF THE STRUCTURAL DRAWINGS, EITHER IN PART OR IN WHOLE, FOR SUBMITTALS OR SHOP DRAWINGS SIGNIFIES ACCEPTANCE OF INFORMATION SHOWN AS CORRECT AND OBLIGES THE USER TO ANY EXPENSE, REAL OR IMPLIED, ARISING FROM THEIR USE.

**GEOTECHNICAL NOTES**

1. STRUCTURAL FILL MATERIAL SHOULD MEET THE GRADATION AND PLASTICITY REQUIREMENTS SET FORTH IN TXDOT STANDARD SPECIFICATIONS 2014; ITEM 247, TYPE A, GRADE 3 OR BETTER.
2. PRIOR TO PLACING FILL MATERIAL REMOVE ALL ORGANIC AND OTHER DELETERIOUS MATERIAL FROM THE EXISTING SUBGRADE FOR A DISTANCE OF 3' 0" BEYOND BUILDING LINE. REMOVE EXISTING MATERIAL IN ORDER TO OBTAIN A MINIMUM OF 6" OF STRUCTURAL FILL BELOW THE SLAB.

**REINFORCING STEEL NOTES**

1. ALL DETAILING OF STEEL REINFORCEMENT AND ACCESSORIES SHALL CONFORM TO ACI COMMITTEE 315 PUBLICATION SP-66, "ACI DETAILING MANUAL."
2. DEFORMED BAR REINFORCEMENT SHALL BE DOMESTIC NEW BILLET STEEL IN CONFORMANCE WITH ASTM A615, BARS - GRADE 60; STIRRUPS, DOWELS - GRADE 40
3. THE SLAB REINFORCING - IS SINGLE LAYER MAT #4@8"
4. WELDING OF REINFORCING STEEL WILL NOT BE PERMITTED UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS.
5. REINFORCING STEEL CLEAR COVER SHALL BE AS FOLLOWS:
  - 1) EARTH-FORMED GRADE BEAMS 1 1/2" TOP, 2" SIDES, 3" BOTTOM
  - 2) SLAB-ON-GRADE AS INDICATED

**STRUCTURAL CONCRETE NOTES**

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 AND ACI 318. ALL CONCRETE SHALL BE LABORATORY DESIGNED AND CONTROLLED.
2. CONCRETE PROTECTION FOR STEEL REINFORCEMENT SHALL BE AS FOLLOWS (SEE ACI 318, SECTION 7.7 FOR CONDITIONS NOT INDICATED). ALL CONCRETE PLACED AGAINST SOIL - SLAB 3" BOTTOM, 2" SIDES, 1 1/2" TOP
3. ALL CONCRETE SURFACES EXPOSED TO THE GROUND MUST BE WATERPROOFED.

**STRUCTURAL WOOD NOTES**

1. ALL WOOD FRAMING SHALL BE KILN-DRIED WITH A MAXIMUM MOISTURE CONTENT AT TIME OF INSTALLATION OF NINETEEN (19) PERCENT AND SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS:
 

MEMBER	MATERIAL	DESIGN PROPERTIES
2x BEAMS, HEADERS, JOISTS, SILL PLATES	#2 GRADE SOUTHERN PINE	Fb = 750 PSI
		Fv = 175 PSI
LAMINATED VENEER LUMBER BEAMS (LVL)	TRUS JOIST 1.9E MICROLLAM LVL	E = 1,400,000 PSI
		Fb = 2,600 PSI
		Fv = 285 PSI
BEARING PLATES, LEDGERS	#3 GRADE SPRUCE-PINE-FIR	Fc = 2,510 PSI
		E = 1,900,000 PSI
		Fb = 500 PSI
		Ft = 250 PSI
WALL STUDS/POST COLUMNS, U.N.O.	STUD GRADE DOUGLAS FIR-LARCH	Fv = 70 PSI
		Fc perp = 425 PSI
		E = 1,200,000 PSI
		Fb = 675 PSI
		Fc = 825 PSI
		E = 1,400,000 PSI
2. SILL PLATES AND OTHER MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED FOR MOISTURE RESISTANCE.
3. THE FLOOR OF THE FIRST LEVEL IS ARRANGED ON THE GROUND.

ALLOWABLE STRESSES ARE UNFACTORED AND ARE BASED ON THE 2018 NATIONAL DESIGN SPECIFICATION, PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.

**WOOD FLOOR NOTES**

1. HOLES AND NOTCHES IN BEAMS AND HEADERS ARE NOT PERMITTED UNLESS VERIFIED IN WRITING BY THE ENGINEER OF RECORD.
2. BEAMS COMPRISED OF TWO (2) MEMBERS OR MORE MEMBERS SHALL BE GLUED AND NAILED TOGETHER WITH A MINIMUM OF TWO (2) ROWS OF 16d NAILS AT TWELVE (12) INCHES ON CENTER. BEAMS COMPRISED OF THREE (3) OR MORE MEMBERS SUPPORTING LOAD THROUGH SIDE HANGERS SHALL HAVE ADDITIONAL 1/2 INCH DIAMETER THRU BOLTS AT EIGHTEEN (18) INCHES ON CENTER, STAGGERED TOP AND BOTTOM. USE 1/2 INCH PLYWOOD OR MEMBERS OF SAME DEPTH AS REQUIRED TO FLUSH OUT WALL.
3. SPLICING OF MEMBERS SHALL NOT BE PERMITTED UNLESS SHOWN ON THE PLANS OR VERIFIED IN WRITING BY THE ENGINEER.
4. INSTALL MEMBERS TRUE, PLUMB AND LEVEL AND PROVIDE ADEQUATE TEMPORARY BRACING AND SHORING UNTIL FINAL CONNECTIONS ARE MADE.

**WOOD CONNECTOR NOTES**

1. NAILS, SPIKES, STAPLES, BOLTS, NUTS, WASHERS, ETC. SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 FOR EXTERIOR AND/OR TREATED WOOD LOCATIONS. PROVIDE PLAIN FINISH FASTENERS FOR INTERIOR LOCATIONS.
2. FRAMING CONNECTORS SHALL BE SIMPSON "STRONG-TIE" OR APPROVED SUBSTITUTE AND SHALL BE BUILDING CODE APPROVED FOR THE TYPE OF INSTALLATION INDICATED. FRAMING CONNECTORS THAT ARE EXPOSED TO EXTERIOR CONDITIONS AND/OR ARE IN CONTACT WITH TREATED WOOD SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 OR FABRICATED WITH A MINIMUM G185 GALVANIZED COATING IN ACCORDANCE WITH ASTM A653. ALL OTHER FRAMING CONNECTORS SHALL FOR SHALL BE FABRICATED WITH A MINIMUM G90 GALVANIZED COATING IN ACCORDANCE WITH ASTM A653.
3. UNLESS NOTED OTHERWISE, SILL PLATES AT THE BUILDING EXTERIOR SHALL BE FASTENED TO THE FOUNDATION WITH GALVANIZED 1/2 INCH DIAMETER, ASTM A307 ANCHOR BOLTS AT FOUR (4) FEET ON CENTER (MINIMUM OF TWO (2) BOLTS PER PLATE). AN ANCHOR BOLT SHALL BE LOCATED NO MORE THAN TWELVE (12) INCHES AND NO LESS THAN FOUR (4) INCHES FROM THE END OF EACH SILL PLATE. ANCHOR BOLTS SHALL BE PLACED WITH HEXAGONAL NUTS AND WASHERS WITH A MINIMUM OUTSIDE DIAMETER OF 1 3/8 INCHES. ANCHOR BOLTS SHALL BE PLACED WITH A MINIMUM OF SIX (6) INCHES OF EMBEDMENT INTO FOUNDATION CONCRETE.

**WOOD TRUSS NOTES**

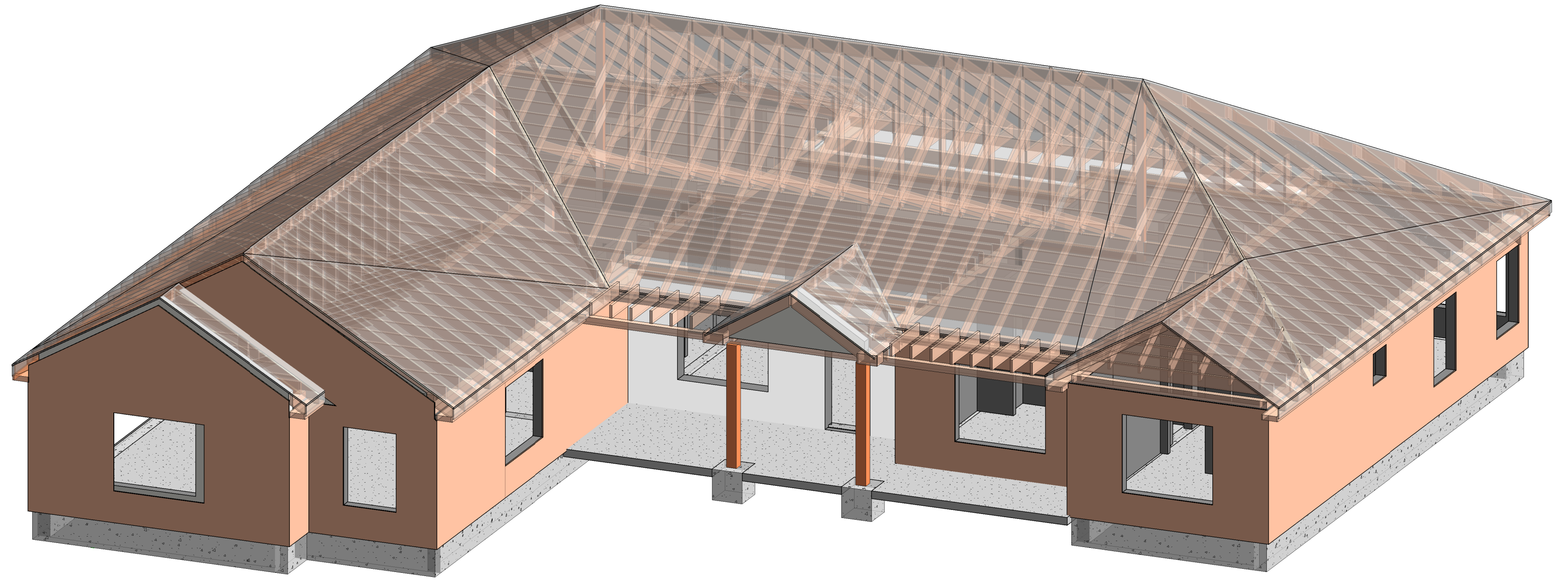
1. DESIGN TRUSSES IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE DESIGN SPECIFICATIONS FOR CONNECTOR PLATES." ALL TRUSSES SHALL BE GRADE STAMPED PER W.C.I.B. RULES.
2. THE CONTRACTOR SHALL COMPLY WITH "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (HB-91) BY THE TRUSS PLATE INSTITUTE DURING THE INSTALLATION OF FLOOR AND ROOF TRUSSES.
3. ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER TO SUPPORT A TOTAL LOAD OF FORTY (40) PSF. COMPOSED OF TWENTY (20) PSF DEAD LOAD (TEN (10) PSF ON THE TOP CHORD AND TEN (10) PSF ON THE BOTTOM CHORD) AND TWENTY (20) PSF LIVE LOAD FOR ALL SPAN CONDITIONS INDICATED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, IN ADDITION, ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER TO SUPPORT ALL SNOW/DRIFT LOADS REQUIRED BY THE BUILDING CODE NOTED ABOVE. AT RTU CONDENSOR AREAS, ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER TO SUPPORT AN ADDITIONAL TOP CHORD LIVE LOAD OF TWENTY (20) PSF. ROOF TRUSS DEFLECTIONS SHALL BE LIMITED TO L/180 FOR TOTAL LOAD AND L/240 FOR LIVE LOAD ONLY.
4. ROOF TRUSSES AND END ANCHORAGES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER FOR A NET UPLIFT OF FIFTEEN (15) PSF.
5. THE CONTRACTOR SHALL SUBMIT COMPLETE TRUSS SHOP DRAWINGS AND DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. SHOP DRAWINGS SHALL INCLUDE FRAMING PLANS SHOWING ALL PREFABRICATED MEMBERS WITH MARK NUMBERS FOR EACH MEMBER TYPE.
6. PROVIDE ANCHORAGE, ERECTION BRACING, AND PERMANENT BRIDGING AS RECOMMENDED BY THE TRUSS MANUFACTURER.
7. AT ROOF TRUSS GIRDERS, PROVIDE ONE (1) STUD BELOW EACH GIRDER SUPPORT FOR EVERY TEN (10) FEET OF TRUSS GIRDER SPAN LENGTH. AS A MINIMUM, PROVIDE TWO (2) STUDS AT EACH SUPPORT. BUILT-UP STUD COLUMNS SHALL BE PROVIDED AT EACH LEVEL AND WITHIN THE FLOOR SYSTEM TO PROVIDE A CONTINUOUS LOAD PATH TO THE FOUNDATION. BUILT-UP STUD COLUMNS SHALL BE NAILED TOGETHER WITH 16d NAILS AT TWENTY (20) INCHES ON CENTER FOR THE FULL STUD HEIGHT.

**WOOD FRAME NOTES**

1. CONSTRUCTION SHALL BE IN CONFORMANCE WITH N.Y.S.B.C.
2. ALL DIMENSIONS & MATERIAL ARE PROVIDED AS A MINIMUM REQUIREMENTS FOR SEISMIC & WIND ANALYSIS; CONTRACTOR TO COORDINATE WITH ARCHITECTURAL DRAWINGS.
3. IF FLOOR HEIGHT IS NOT EVEN WITH THE PANEL WIDTH (IF PANELS APPLIED HORIZONTALLY) PROVIDE ADDITIONAL BLOCKING AT PANEL EDGES TO ACHIEVE REQUIRED FASTENER SPACING.
4. ALL SHEAR WALLS TO BE ANCHORED TO THE FOUNDATION WALL/BASEMENT WALL/FOOTING.
5. PANEL SHEATHING JOINTS IN SHEAR WALLS SHALL OCCUR OVER STUDS OR BLOCKING. ADJACENT PANEL SHEATHING JOINTS SHALL OCCUR OVER AND BE NAILED TO COMMON FRAMING MEMBER.
6. END JOINTS OF ADJACENT COURSES OF PANEL SHEATHING SHALL NOT OCCUR OVER THE SAME STUD (PROVIDE BRICK PATTERN).

**WOOD FRAME CONNECTOR NOTES**

- 1.1. ALL SHEAR WALLS TO BE TIED WITH ANCHOR BOLTS TO THE FOUNDATION WALL/BASEMENT WALL/FOOTING. SEE FLOOR PLAN FOR LOCATION OF SHEAR WALLS.
2. DRIVE ONE STUD NAIL AT AN ANGLE THROUGH THE STUD INTO THE PLATE.
3. PROVIDE SP2 CONNECTORS ON A SAME STUD WHICH HAS CS16 CONNECTORS TO CREATE CONTINUOUS LOAD PATH.
4. NAILS ARE NOT REQUIRED IN CLEAR SPAN AS PER MANUFACTURER'S SPECIFICATIONS. G.C./SUB CAN PROVIDE ADDITIONAL NAILS IF DESIRED.



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**GENERAL NOTES**

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SHEET №



FOOTING REINFORCING SCHEDULE		
MARKS	LENGTH X WIDTH X DEPTH	REINFORCING
F1	CONT X 10" X 24"	(6)BARS#7; STIRRUPS #4 @12"; DOWEL#7 @8"
F2	CONT X 10" X 24"	(6)BARS#7; STIRRUPS #4 @12"; (2)DOWEL#7 @8"

SLAB REINFORCING		
MARKS	REINFORCING	THICKNESS
SF1	REBAR MAS #4 @8"	6"

1 FOUNDATION PLAN  
3/16" = 1'-0"

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FOUNDATION  
PLAN

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WALL  
PLAN

Author

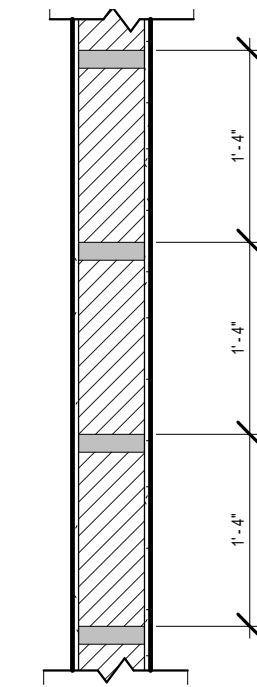
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MARKS	DRAWING LEGEND
	<b>W1.2X6 EXTERIOR MASONRY:</b> - 1/2" GYP. BD. INT. SIDE - 2"X6" STUDS VERIFY SPACING W/ ENGINEER - 1" INSULATION, 1" AIR, 4 1/2" MASONRY ON THE EXTERIOR
	<b>W2.2X6 EXTERIOR SIDING:</b> - 1/2" GYP. BD. INTERIOR SIDE - 2X6 STUDS VERIFY SPACING W/ ENGINEER - 1" INSULATION, 1/2" FINISH ON THE EXTERIOR
	<b>W3.2X6 INTERIOR:</b> - 1/2" GYP. BD. BOTH SIDES - 2X6 STUDS VERIFY SPACING W/ ENGINEER
	<b>W4.2X4 INTERIOR:</b> - 1/2" GYP. BD. BOTH SIDES - 2X4 STUDS VERIFY SPACING W/ ENGINEER
	<b>W5.2X4(6) 1-HR FIREPROOF WAL:</b> - 5/8" FIRE RATED GYP. BD. BOTH SIDES - 2X4(2X6) STUDS VERIFY SPACING W/ ENGINEER
	<b>POST IN WALL</b> MIN.(3)2X DF N.1
	<b>COLUMN</b> - C1 (5 1/2" X 5 1/2") - C2 (8" X 8")

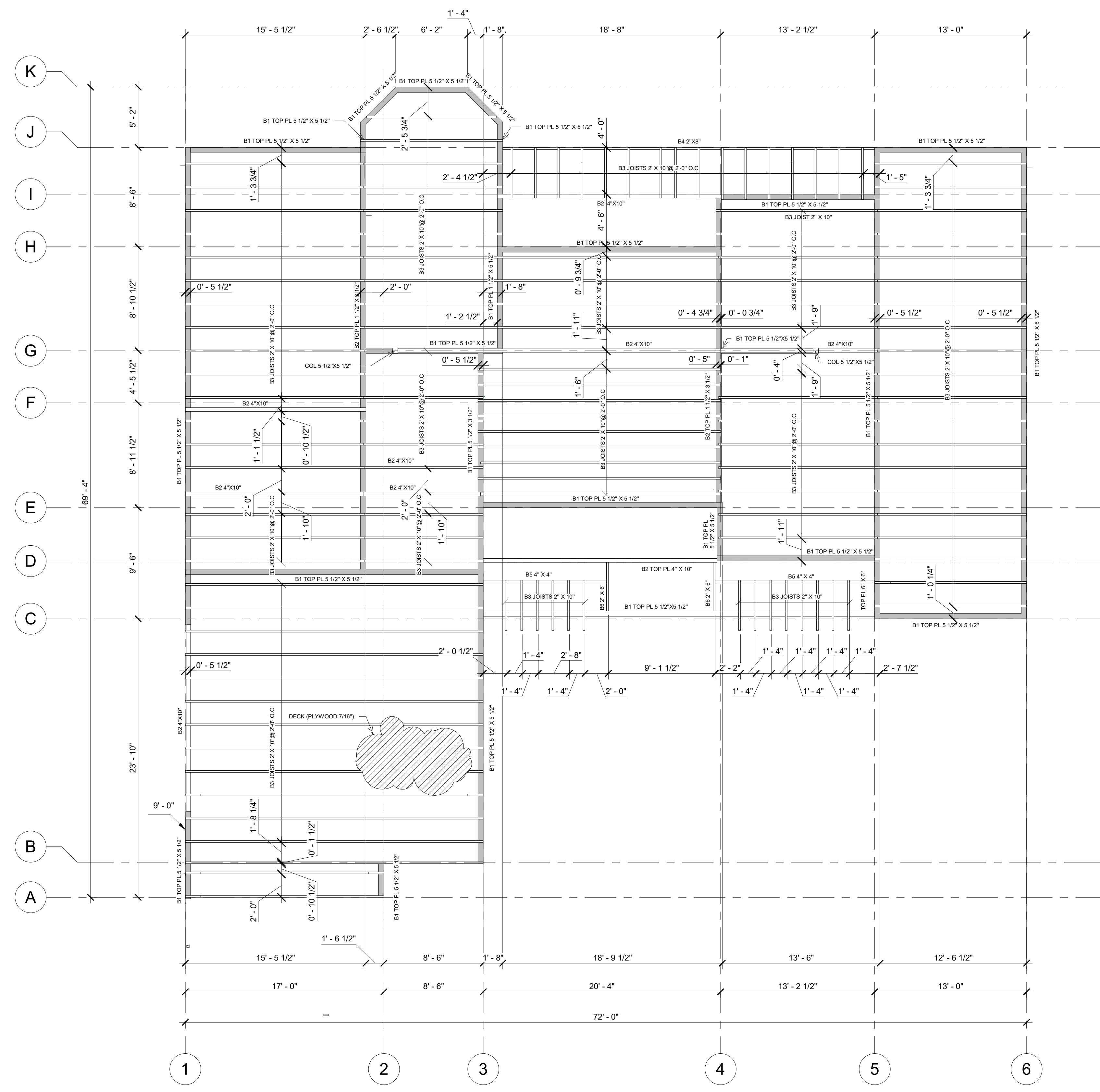


② TYPICAL FRAMING  
3/4" = 1'-0"



① WALL & COLUMN PLAN  
3/16" = 1'-0"

STRUCTURAL FRAMING SCHEDULE		
MARKS	PROFILE	QUANTITY
	4" X 10"	6
B1	6" X 6"	30
B2	4" X 10"	6
B3	2" X 10"	130
B4	2" X 8"	1
B5	4" X 4"	2
B6	2" X 6"	2



1 FLOOR FRAMING  
3/16" = 1'-0"

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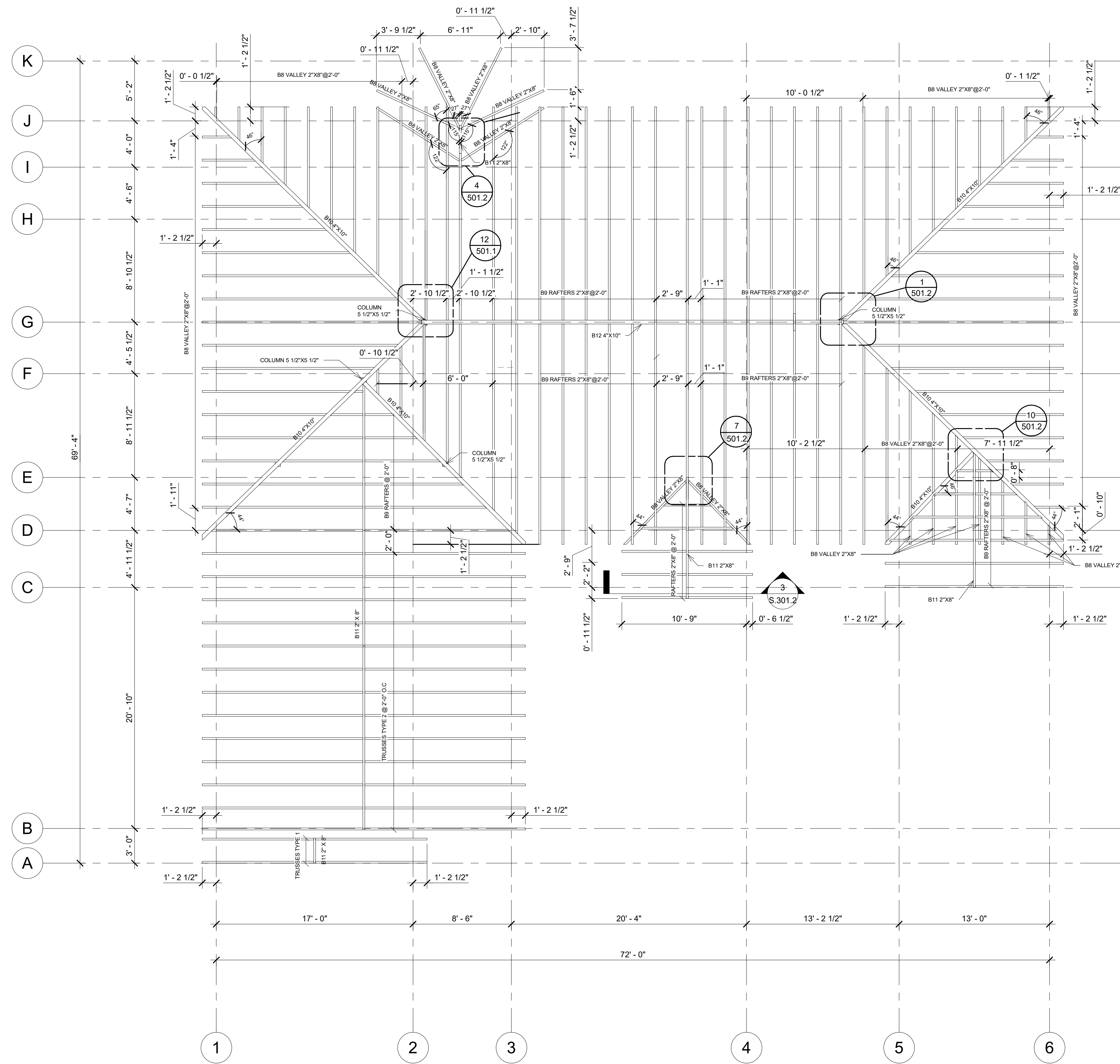
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FLOOR FRAMING PLAN

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STRUCTURAL ROOF FRAMING SCHEDULE		
MARKS	PROFILE	QUANTITY
B8	2" X 8"	83
B9	2" X 8"	38
B10	4" X 10"	6
B11	2" X 8"	4
B12	2" X 8"	1

1 ROOF FRAMING PLAN  
 3/16" = 1'-0"

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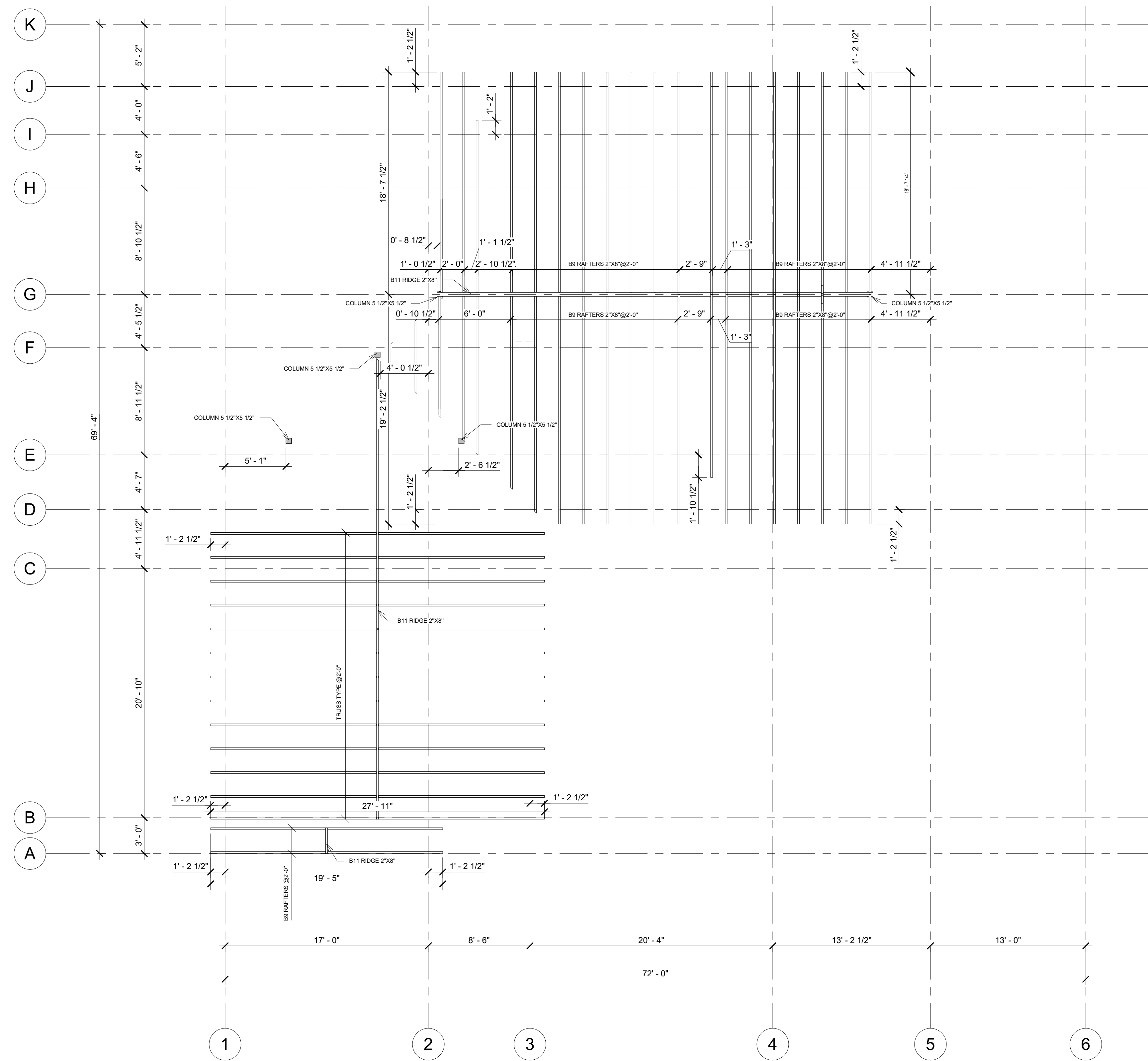
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ROOF  
 FRAMING  
 PLAN

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 Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

**S.104.1**

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1 ROOF FRAMING PLAN STEP 1  
 3/16" = 1'-0"

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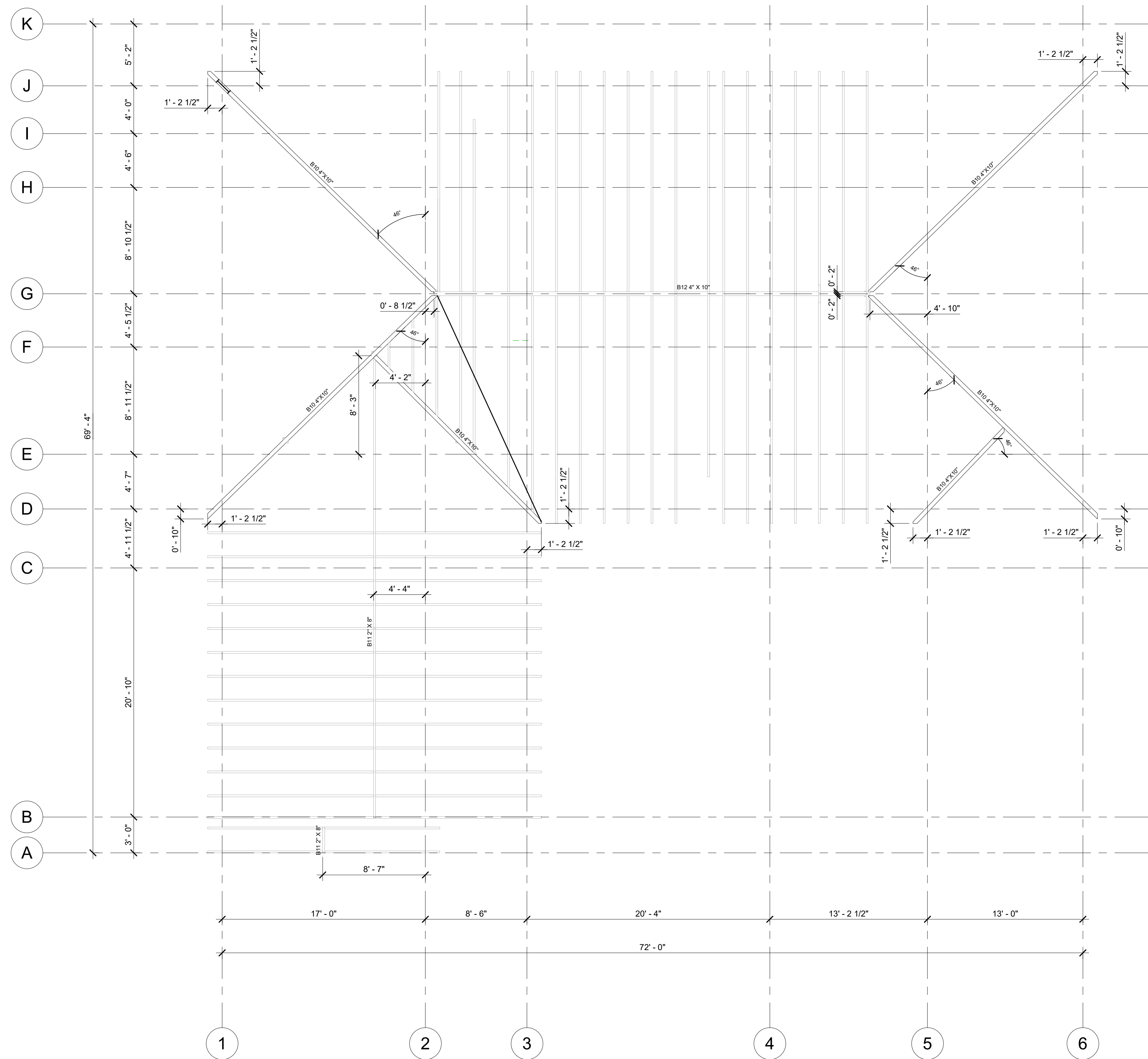
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1-st STAGE  
 OF  
 INSTALLATION

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 Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

**S.104.2**

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1 ROOF FRAMING PLAN STEP 2  
 3/16" = 1'-0"

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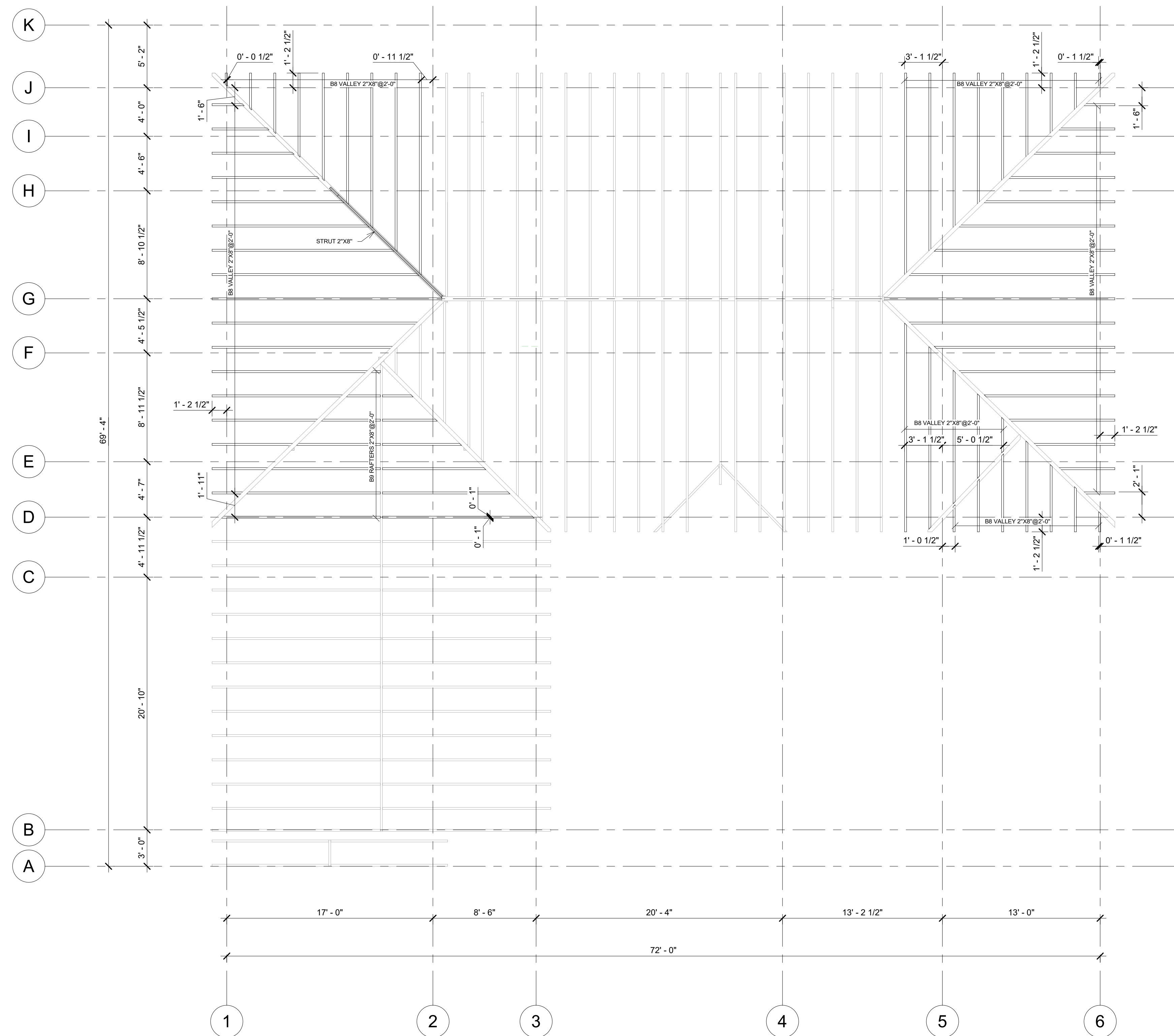
2-nd  
 STAGE OF  
 INSTALLATION

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 Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

**S.104.3**

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1 ROOF FRAMING PLAN STEP 3  
 3/16" = 1'-0"

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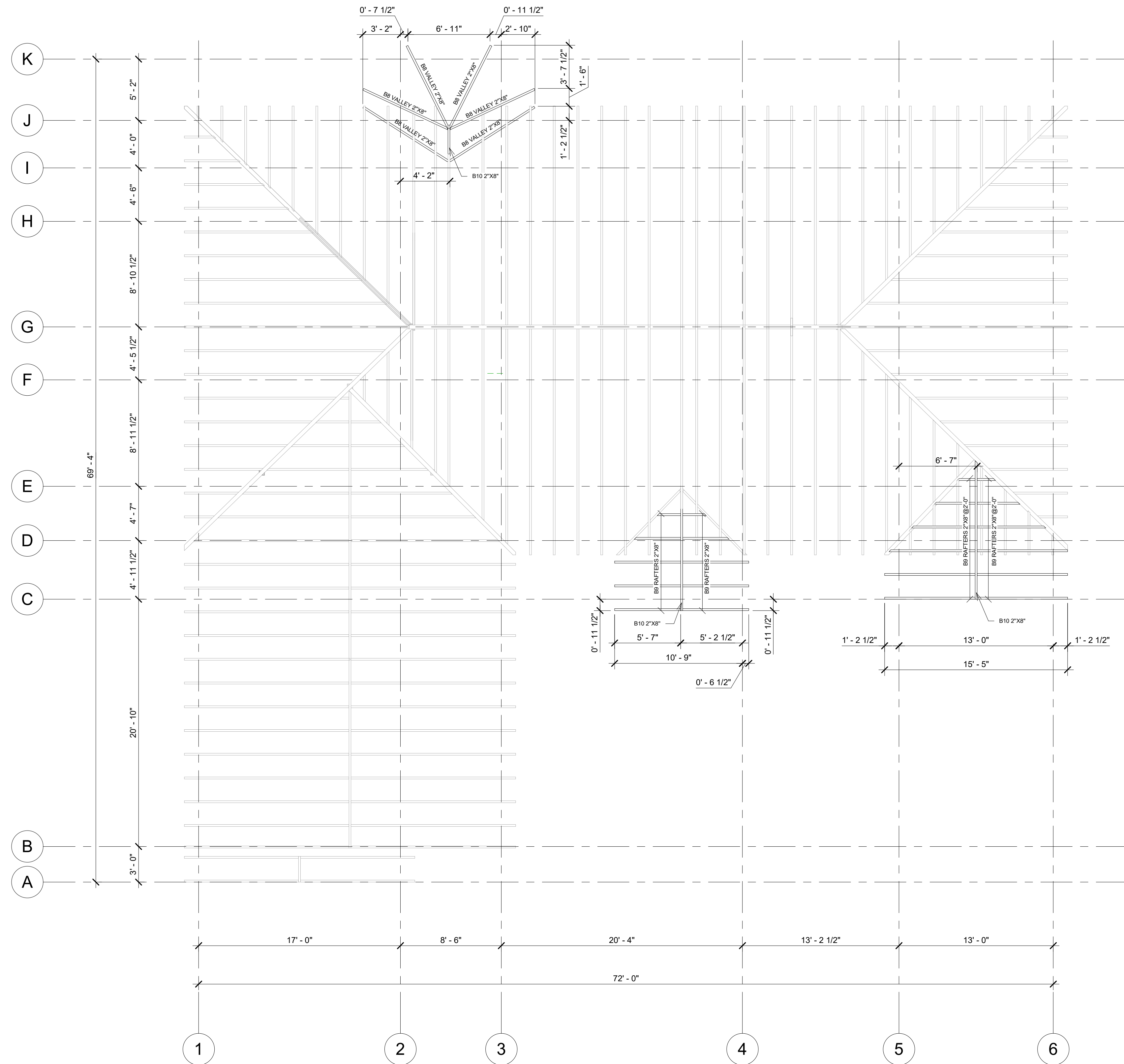
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3-th STAGE  
 OF  
 INSTALLATION

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 Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

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1 ROOF FRAMING PLAN STEP 4  
 3/16" = 1'-0"

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4-th STAGE  
 OF  
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 Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

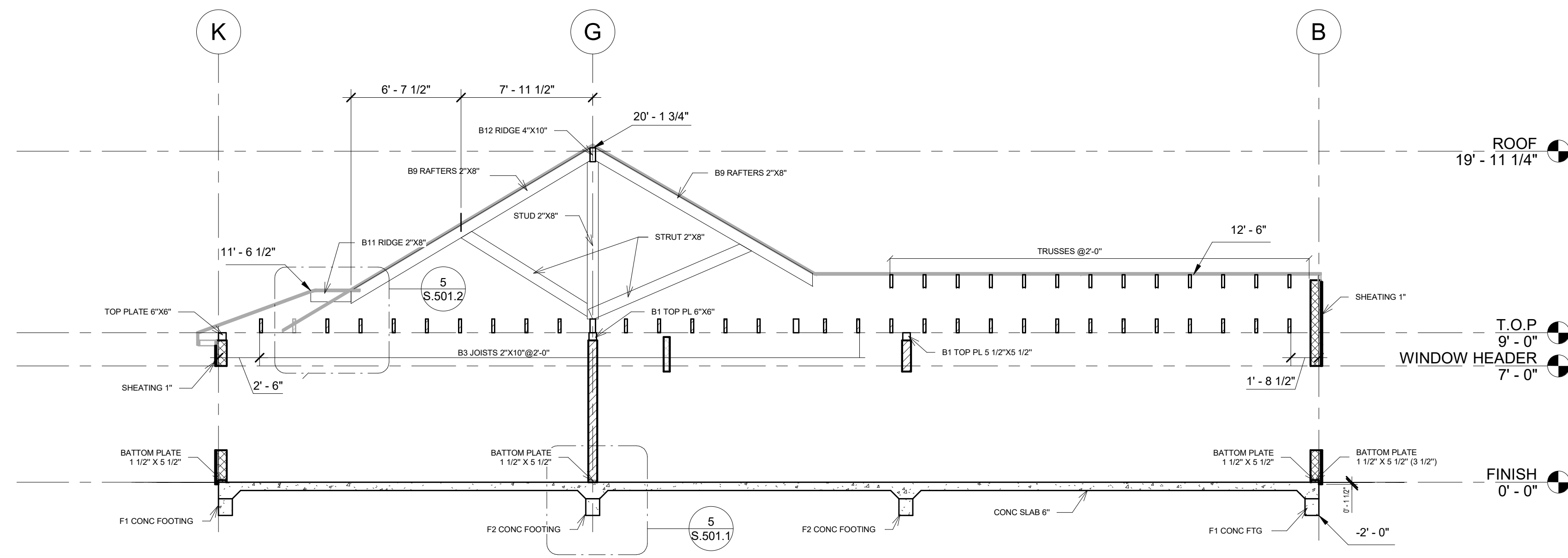
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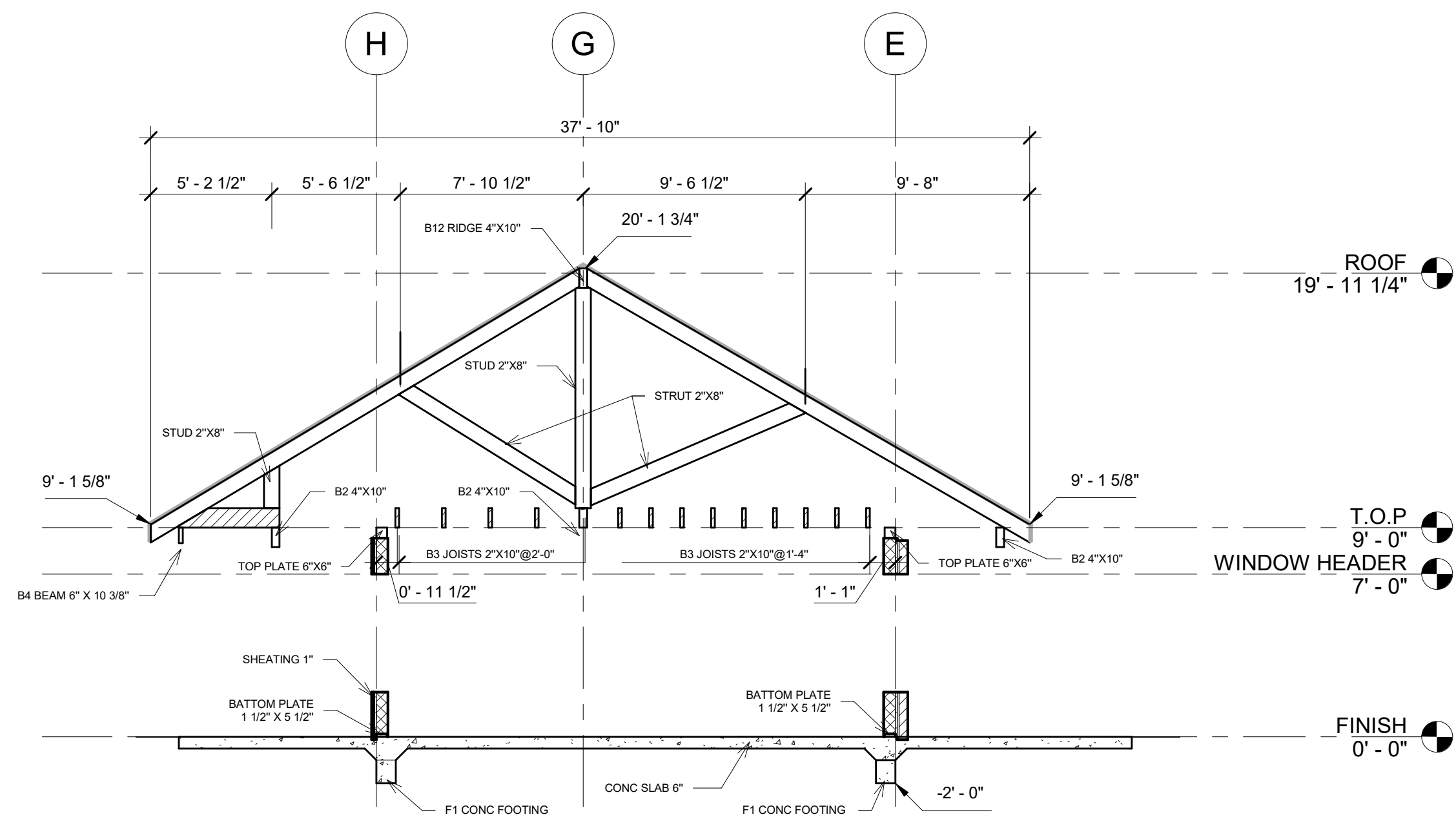


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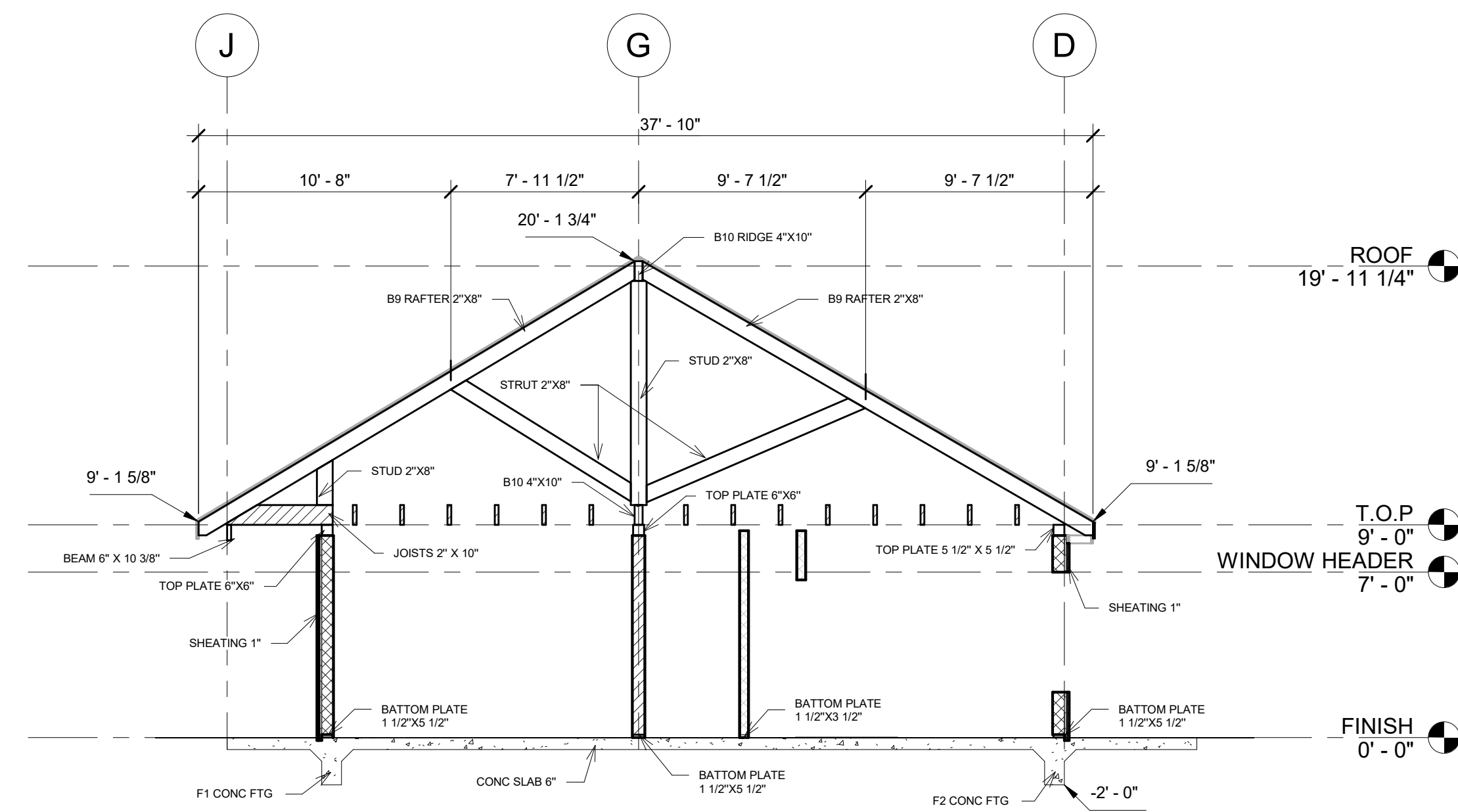
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2 CONSTRUCTION SECTION 1  
3/16" = 1'-0"



3 CONSTRUCTION SECTION 2  
3/16" = 1'-0"



1 CONSTRUCTION SECTION 3  
3/16" = 1'-0"

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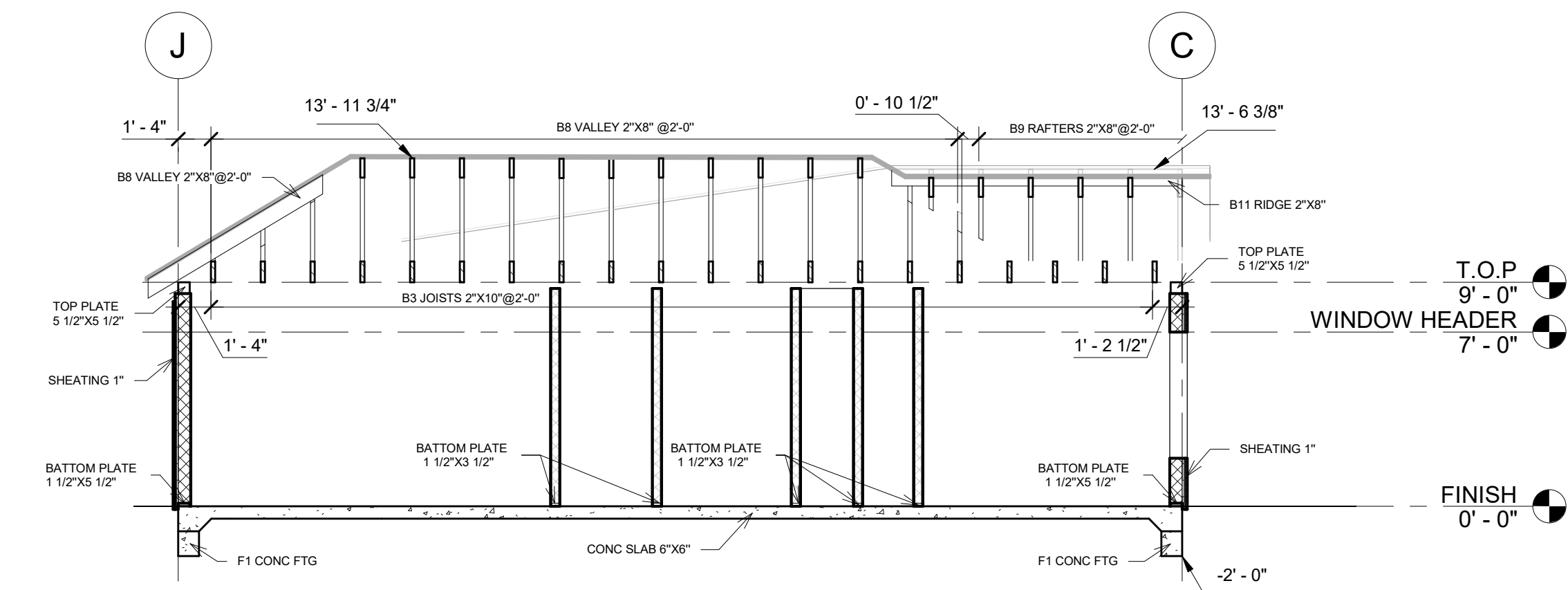
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CONSTRUCTION  
SECTION

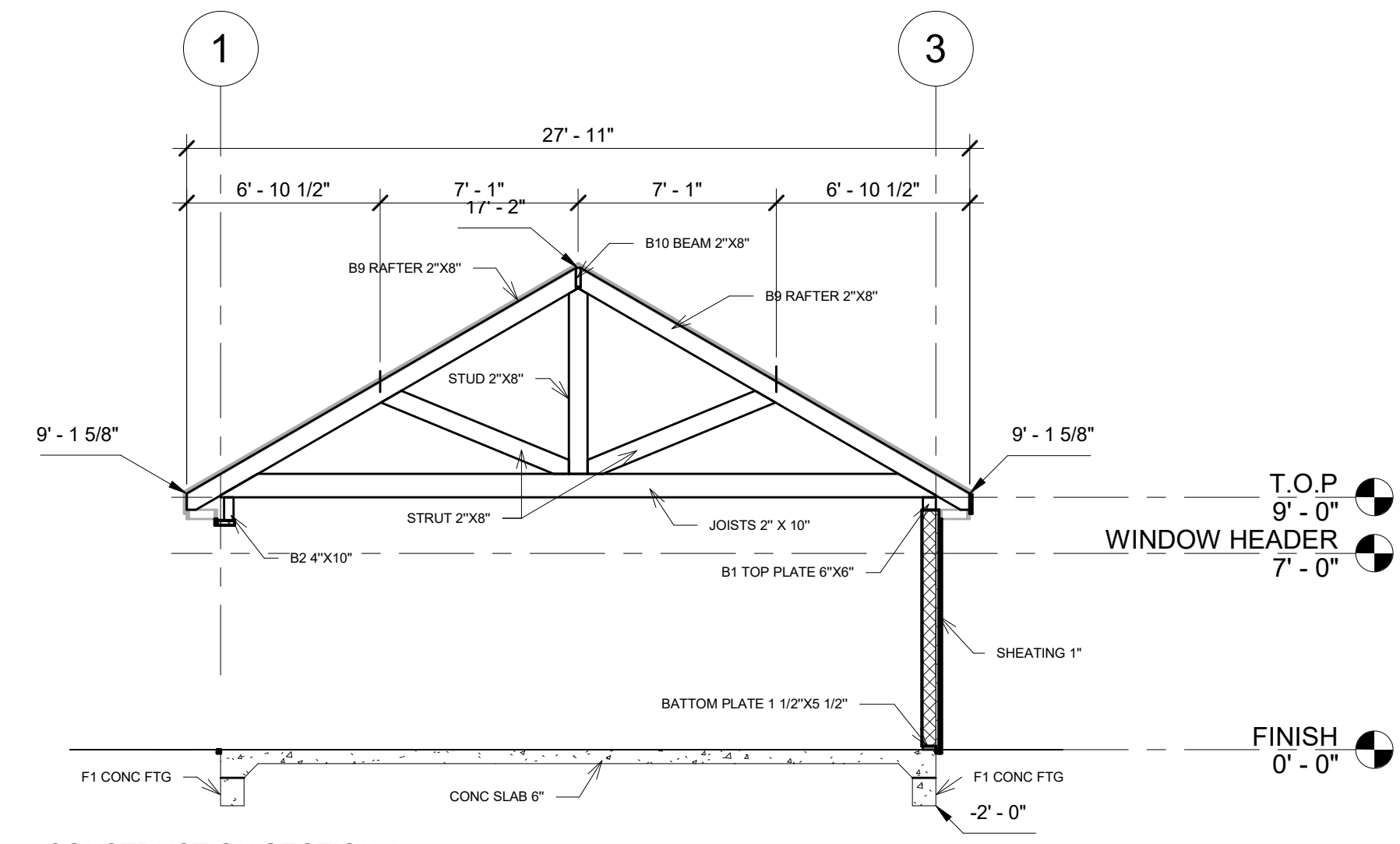
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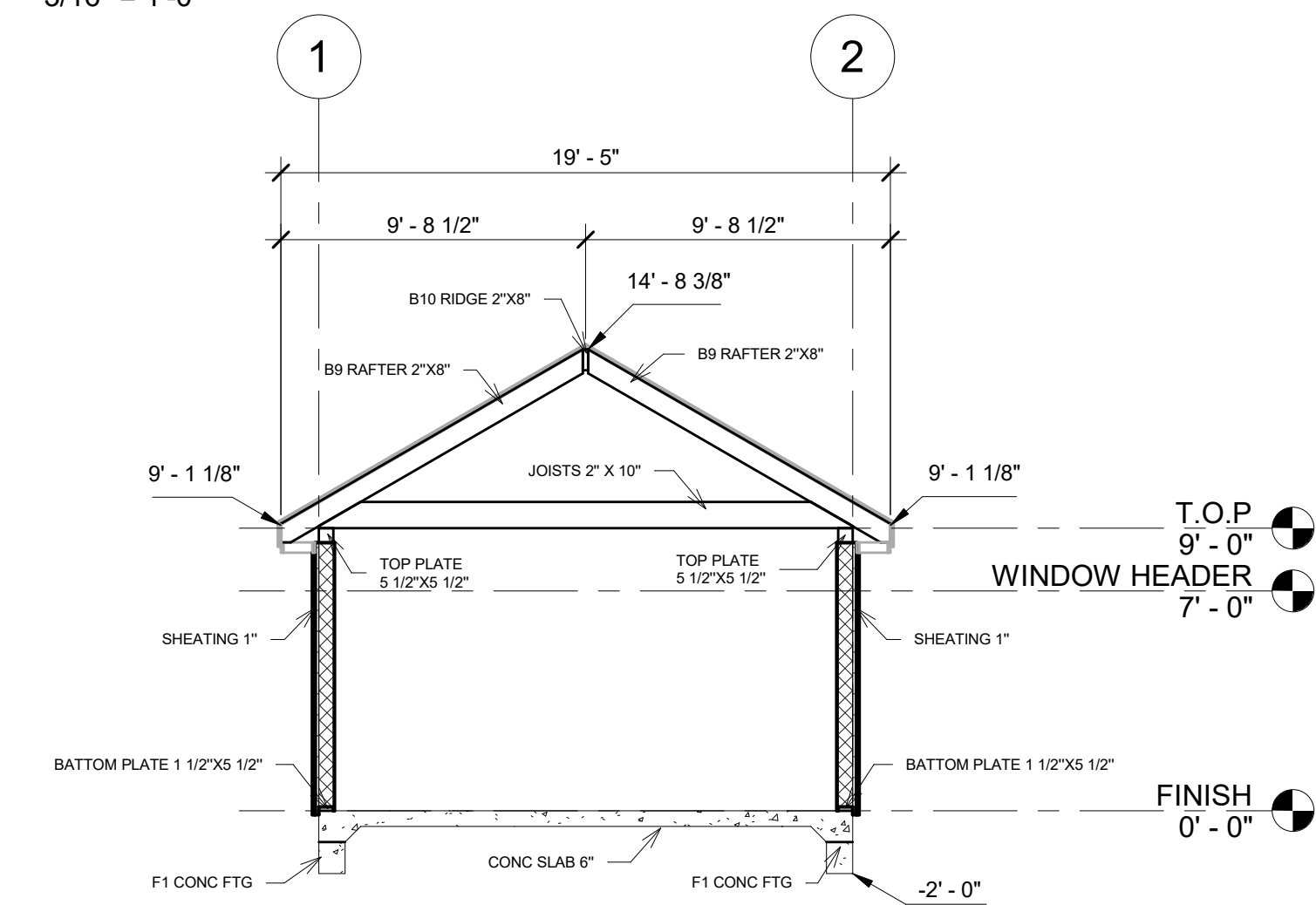
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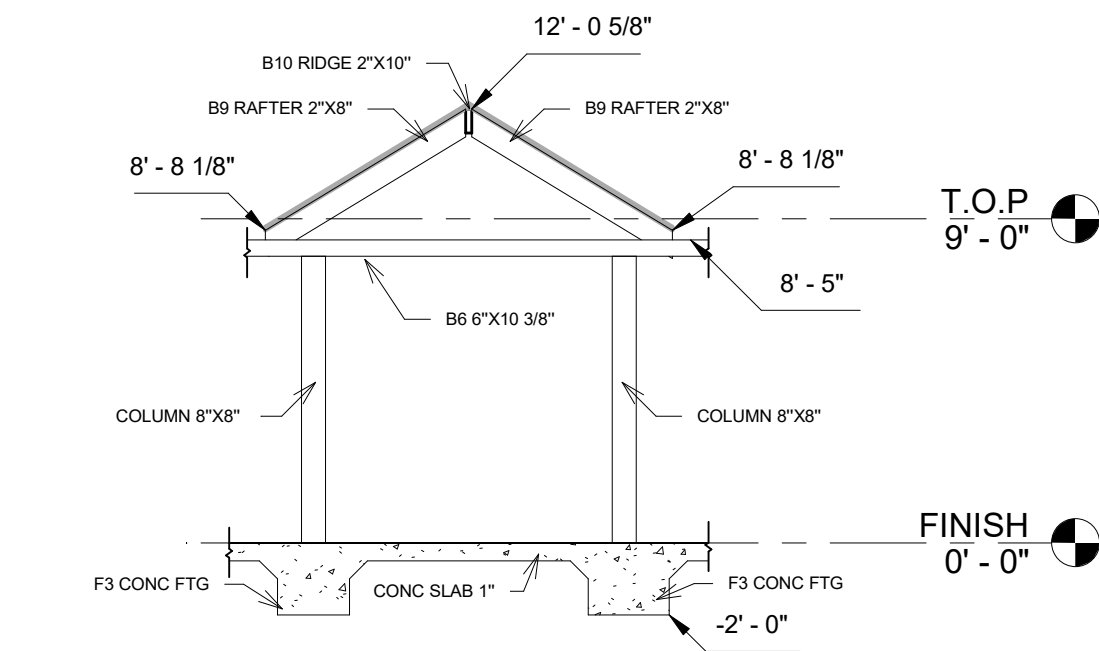
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3/16" = 1'-0"



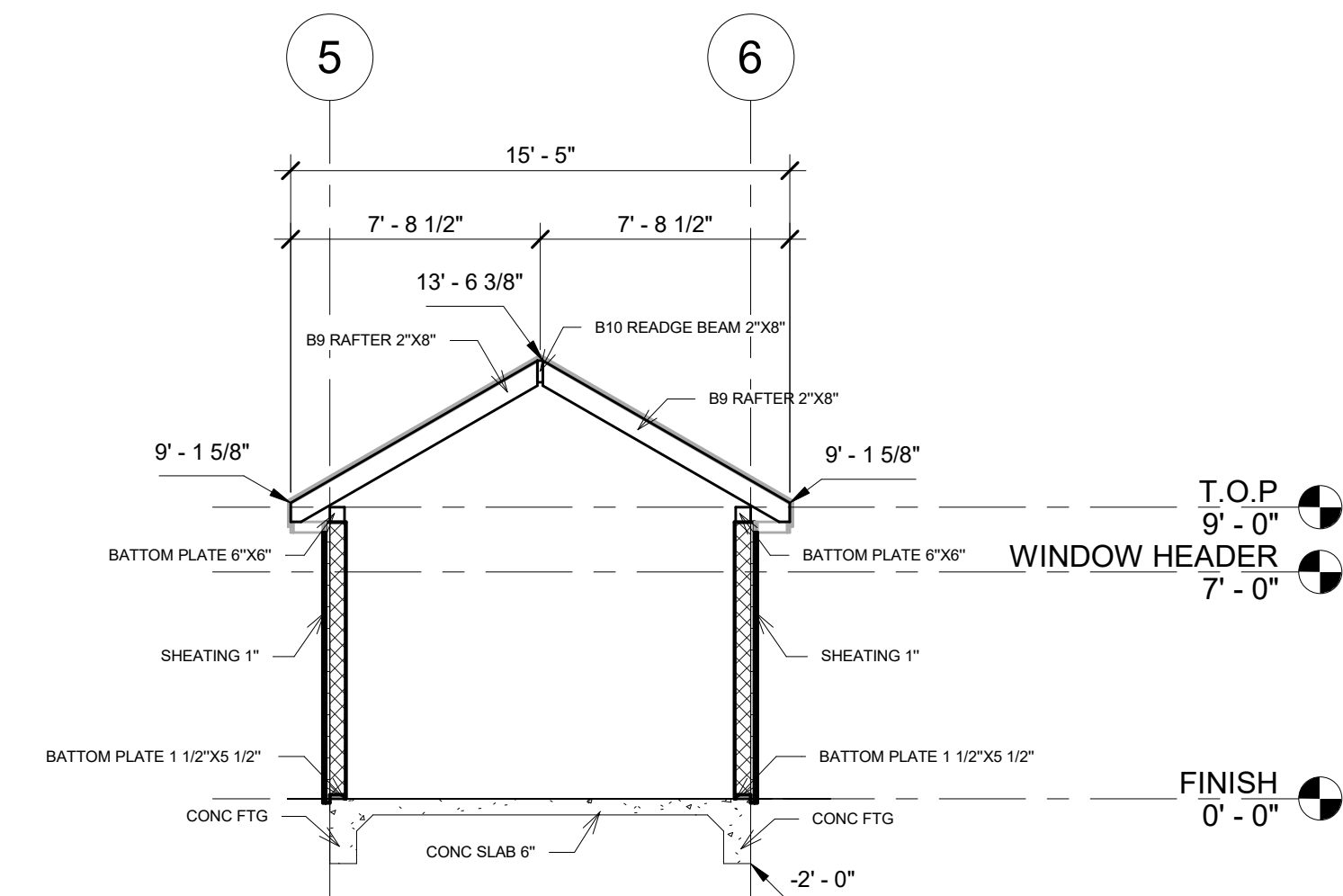
2 CONSTRUCTION SECTION 6  
3/16" = 1'-0"



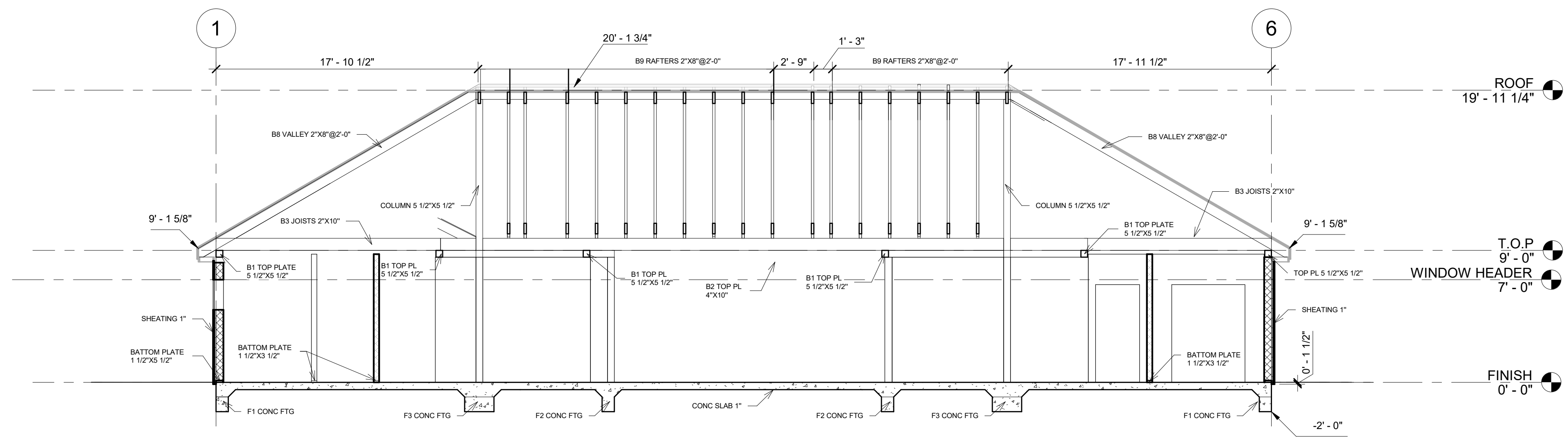
1 CONSTRUCTION SECTION 5  
3/16" = 1'-0"



3 CONSTRUCTION SECTION 7  
3/16" = 1'-0"



4 CONSTRUCTION SECTION 8  
3/16" = 1'-0"



6 CONSTRUCTION SECTION 9  
3/16" = 1'-0"

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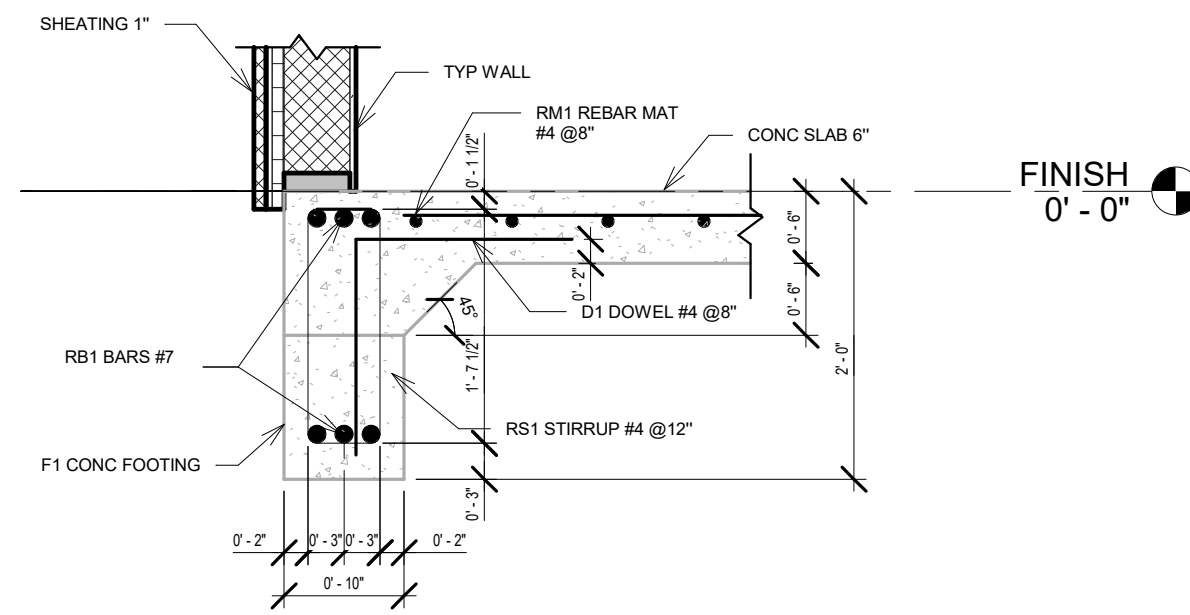
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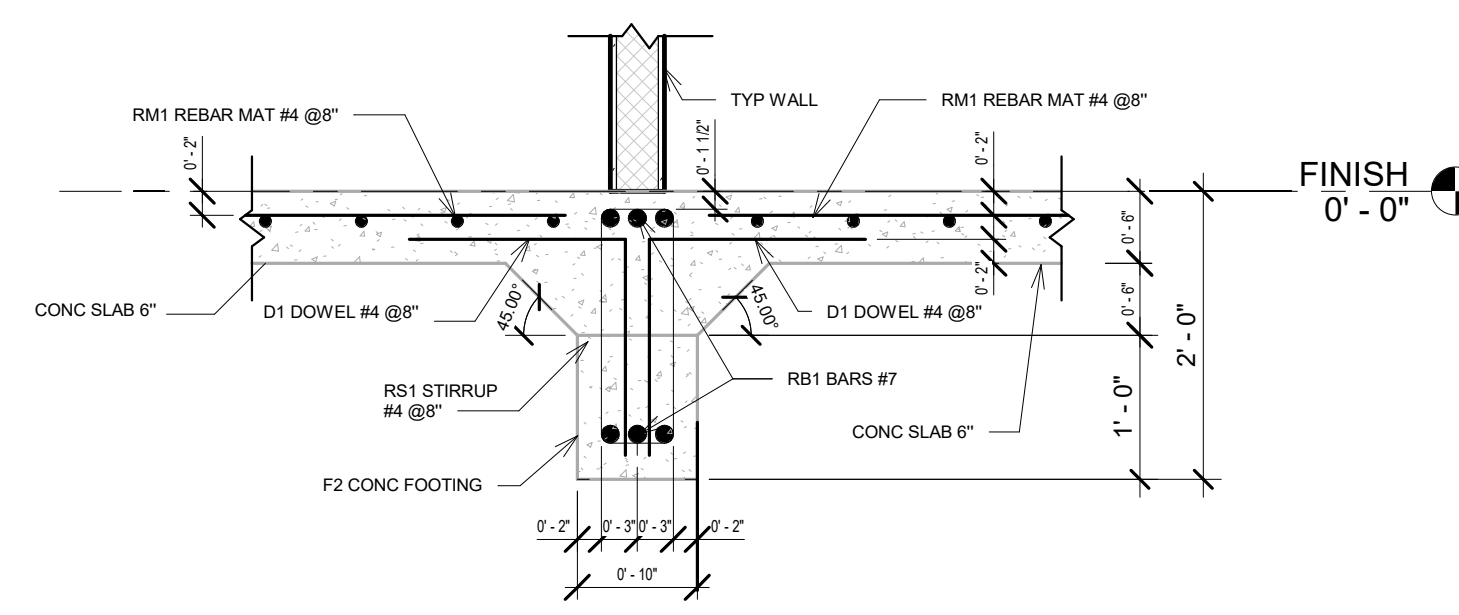
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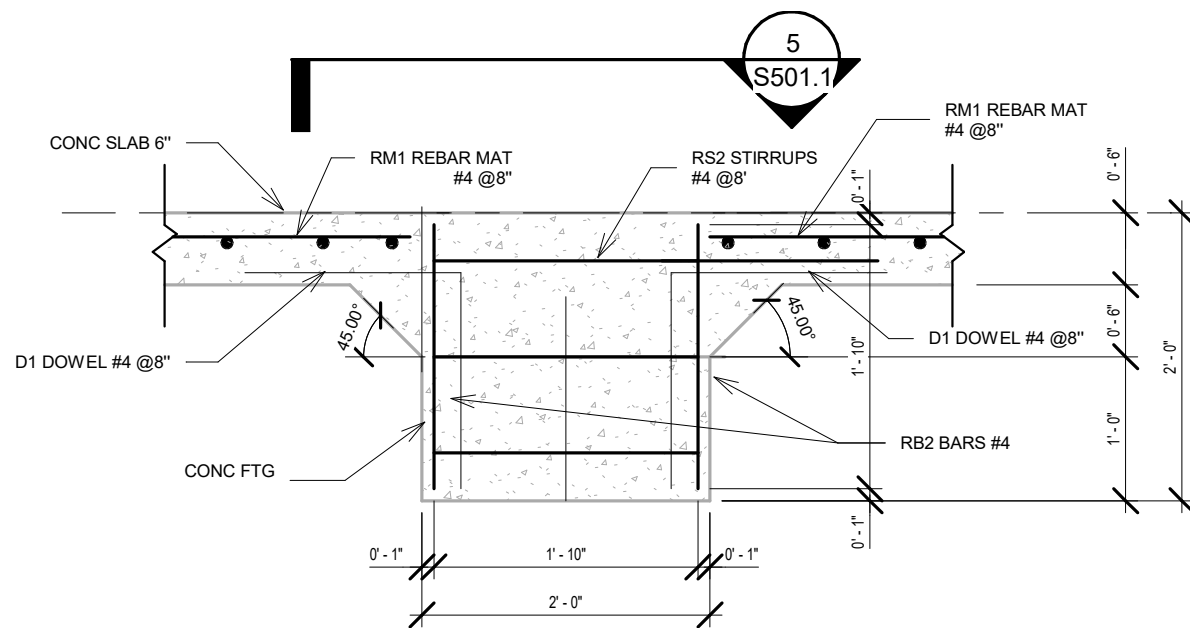
REINFORCING SCHEDULE					
MARKS	QUANTITY	REINFORCING	LENGTH	SPACING	REMARKS
RB1	6	#7GR60	CONT	-	REBAR BARS
RB2	4	#7GR60	CONT	-	REBAR BARS
RS1	-	#4GR40	-	8"	REBAR STIRRUPS
RS2	-	#4GR40	-	8"	REBAR STIRRUPS
D1	-	#4GR40	1'6"X1'6"	8"	DOWELS
RM1	-	#4GR40	-	8"	



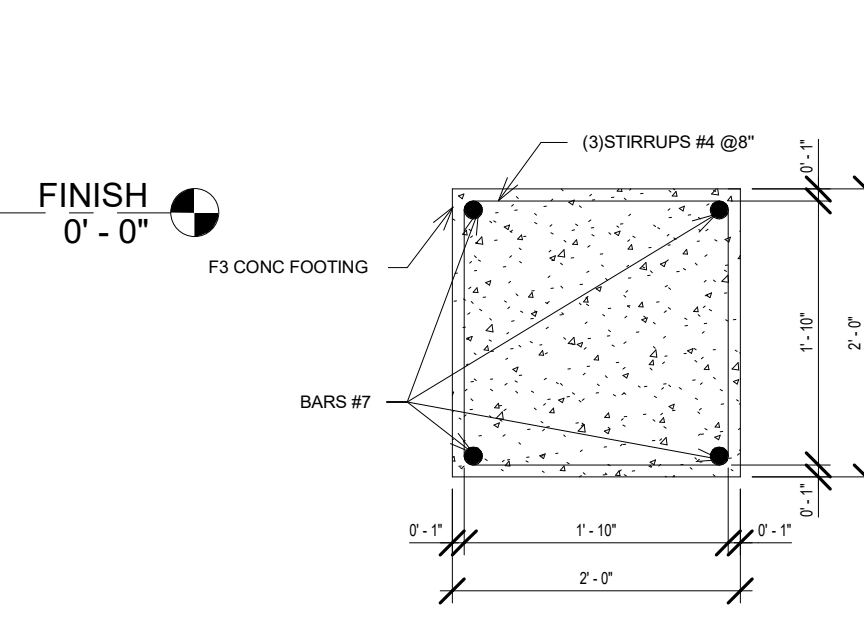
1 FOUNDATION FRONT 1  
3/4" = 1'-0"



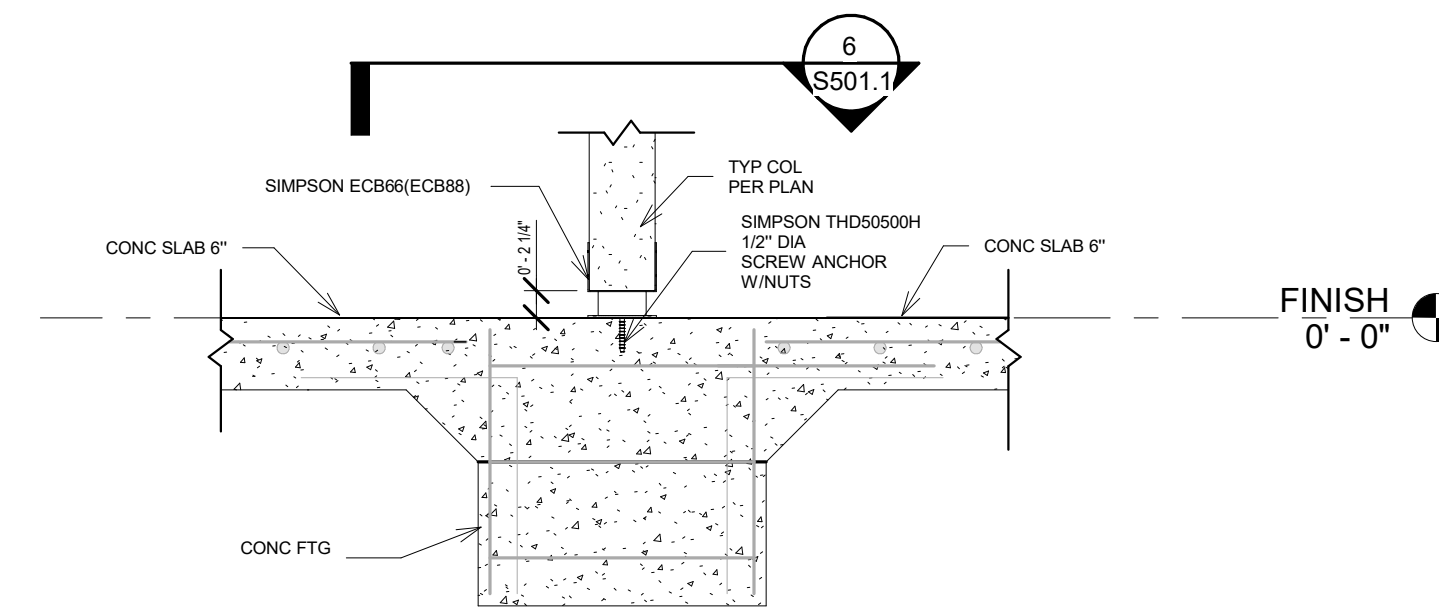
2 FOUNDATION FRONT 2  
3/4" = 1'-0"



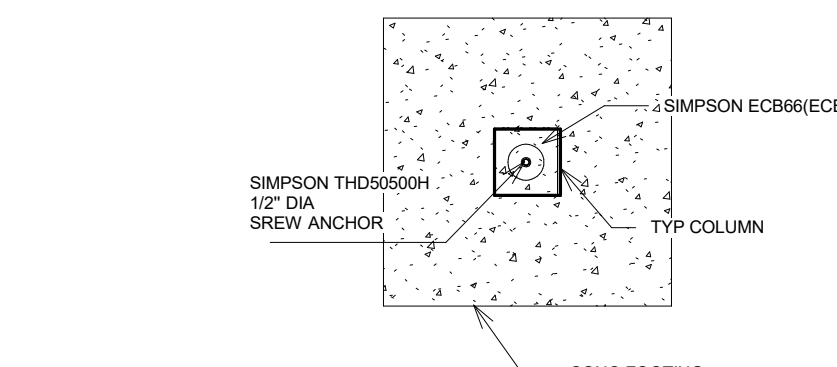
3 FOUNDATION FRONT 3  
3/4" = 1'-0"



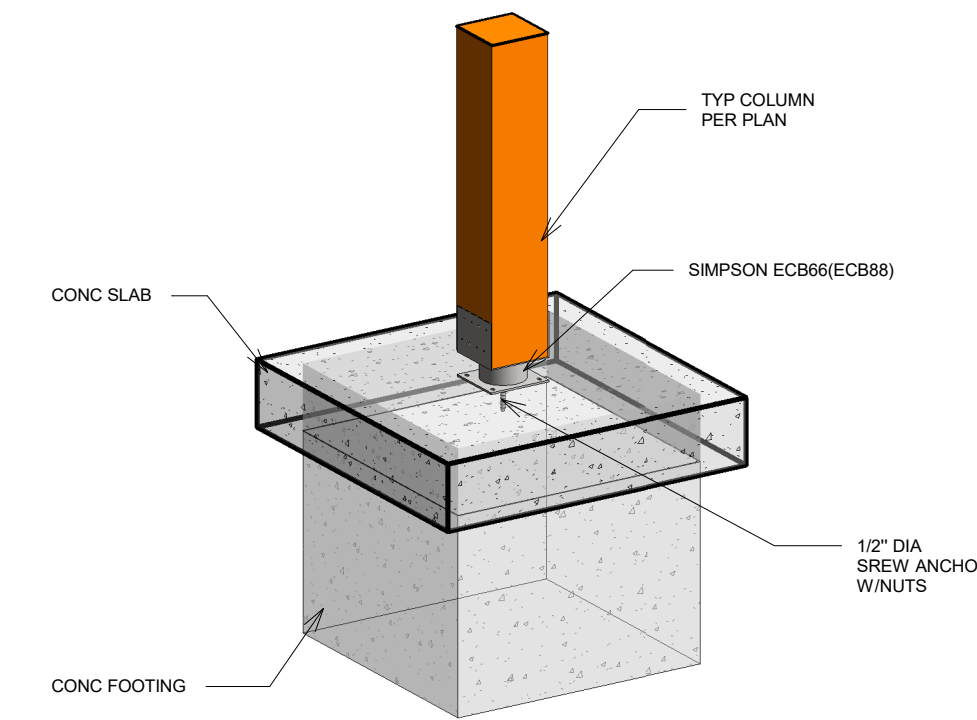
4 FOUNDATION TOP 3  
3/4" = 1'-0"



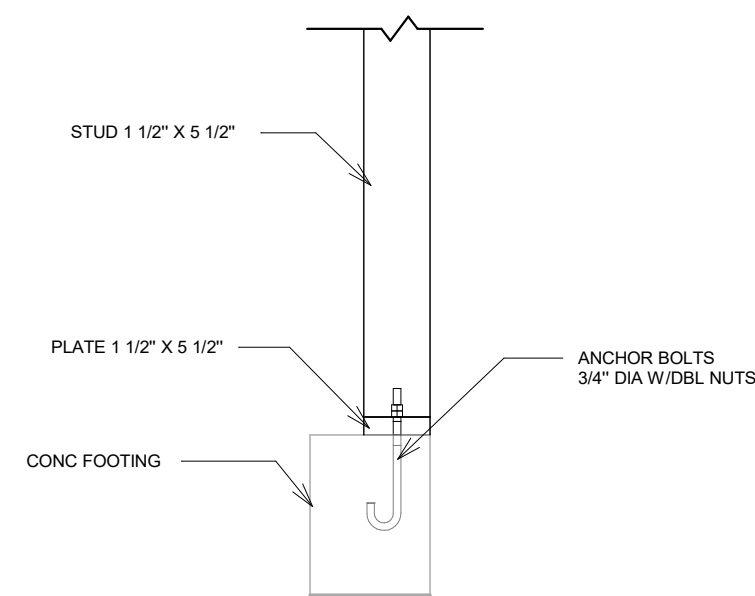
5 COLUMN BASE FRONT  
3/4" = 1'-0"



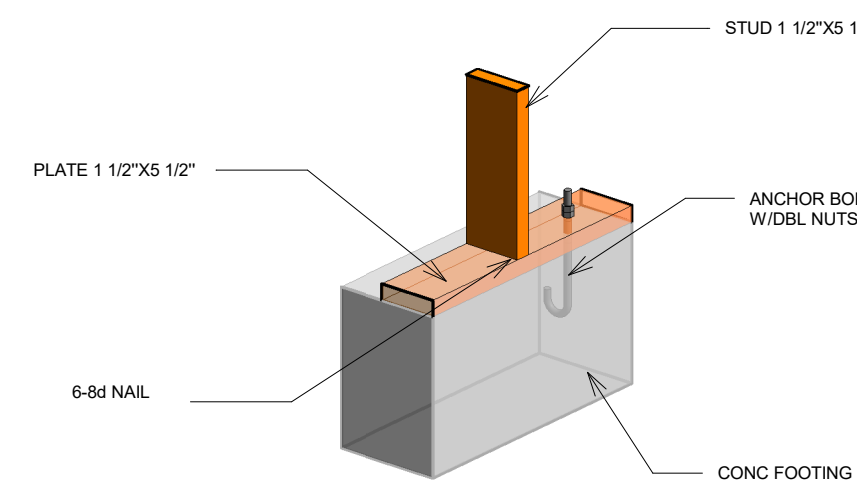
6 COLUMN TOP  
3/4" = 1'-0"



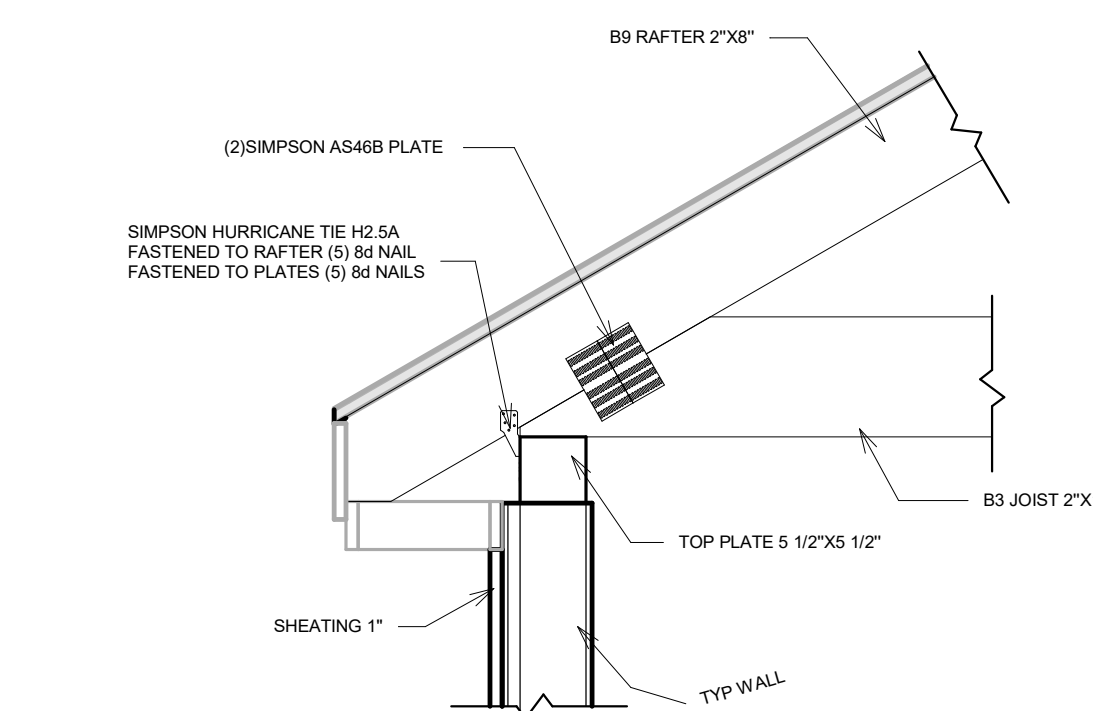
7 COLUMN BASE 3D  
3/4" = 1'-0"



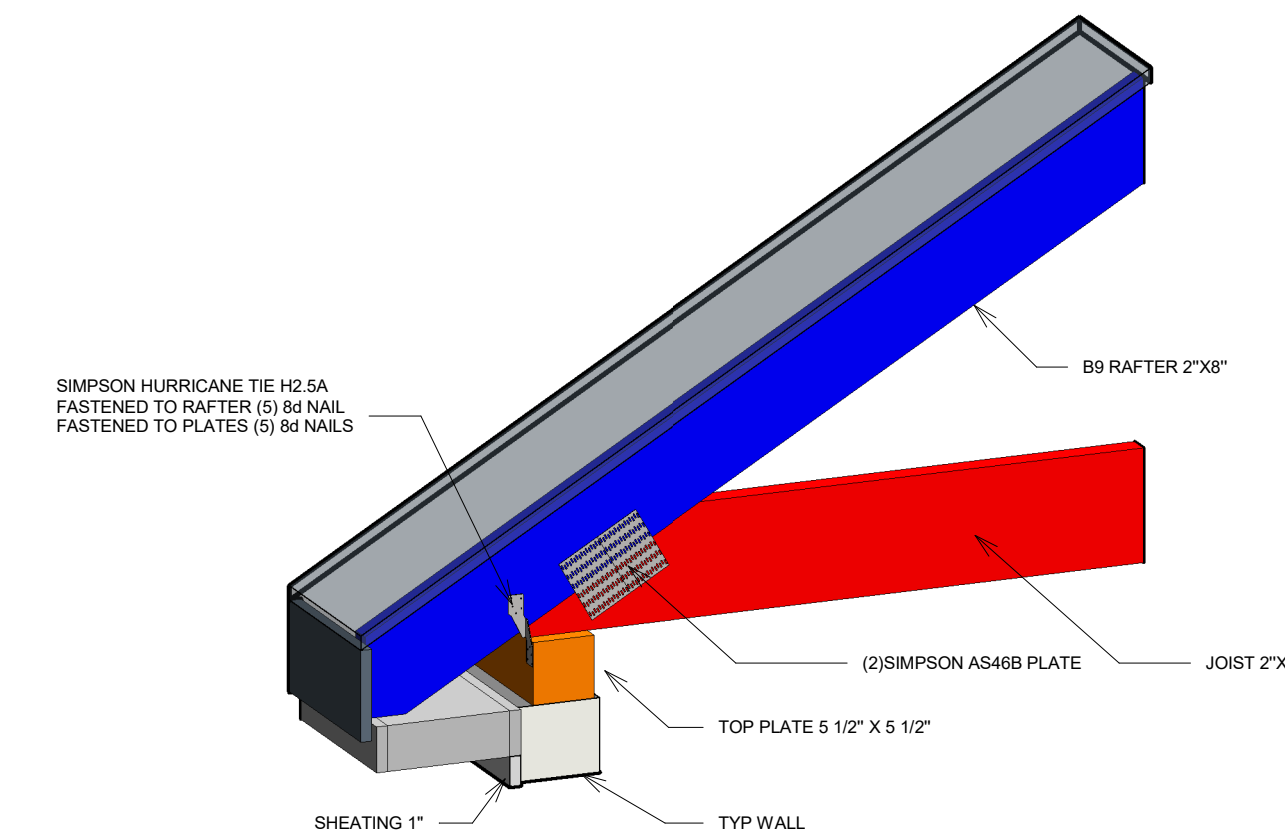
8 CONN FRAMING TO FOUNDATION FRONT  
3/4" = 1'-0"



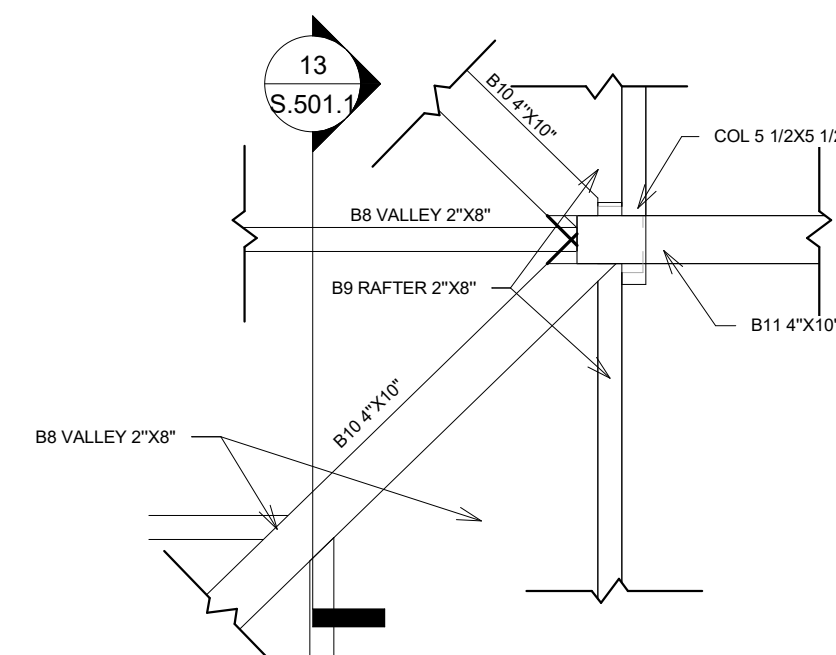
9 CONN FRAMING TO FOUNDATION 3D  
3/4" = 1'-0"



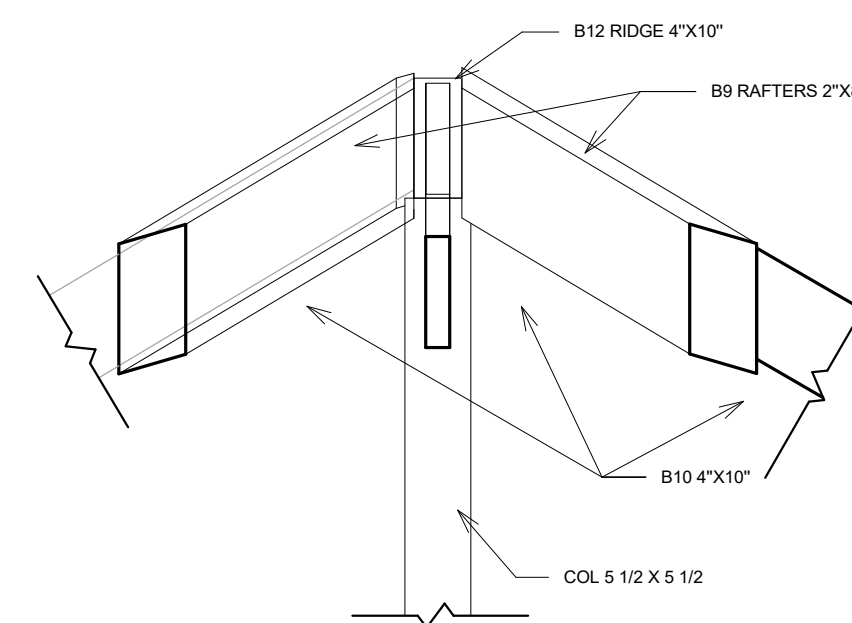
10 ROOF TRUSS CONNECTION FRONT  
3/4" = 1'-0"



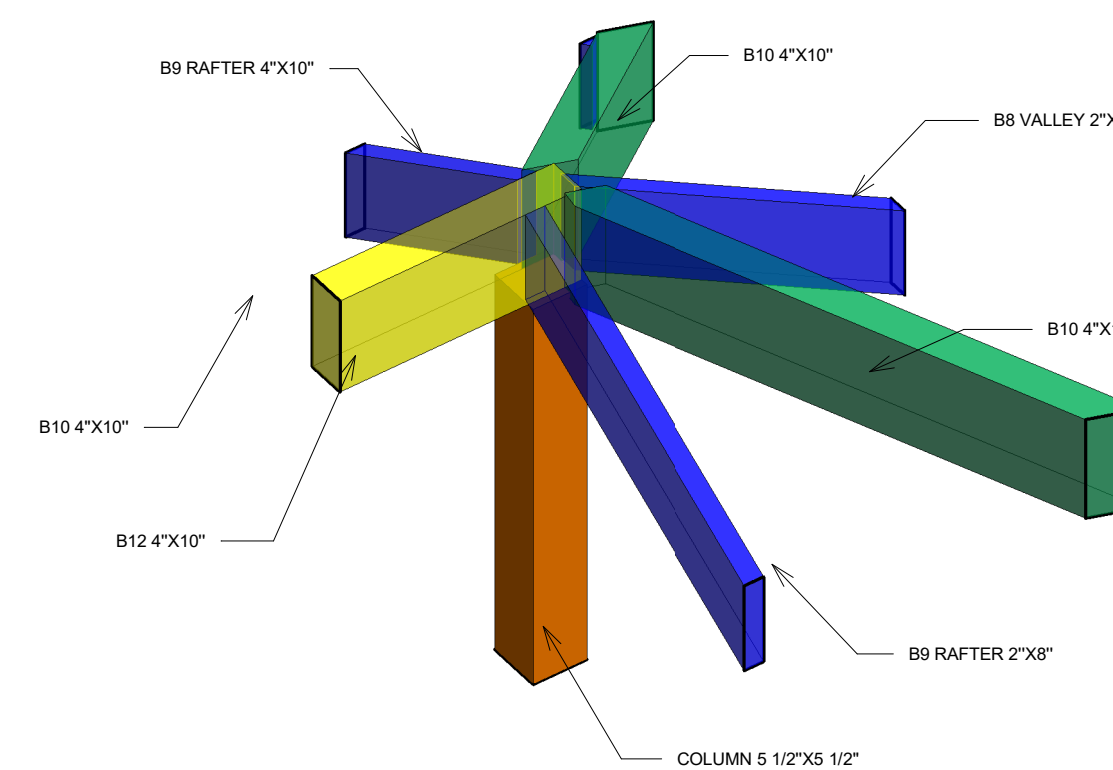
11 ROOF TRUSS CONNECTION 3D  
3/4" = 1'-0"



12 POINT 2 TOP  
3/4" = 1'-0"



13 POINT 2 FRONT  
3/4" = 1'-0"



14 POINT 2 3D  
3/4" = 1'-0"

T2T - DUSTIN WEBER

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TYPICAL  
CONSTRUCTION  
DETAILS

Author: \_\_\_\_\_  
Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

S.501.1

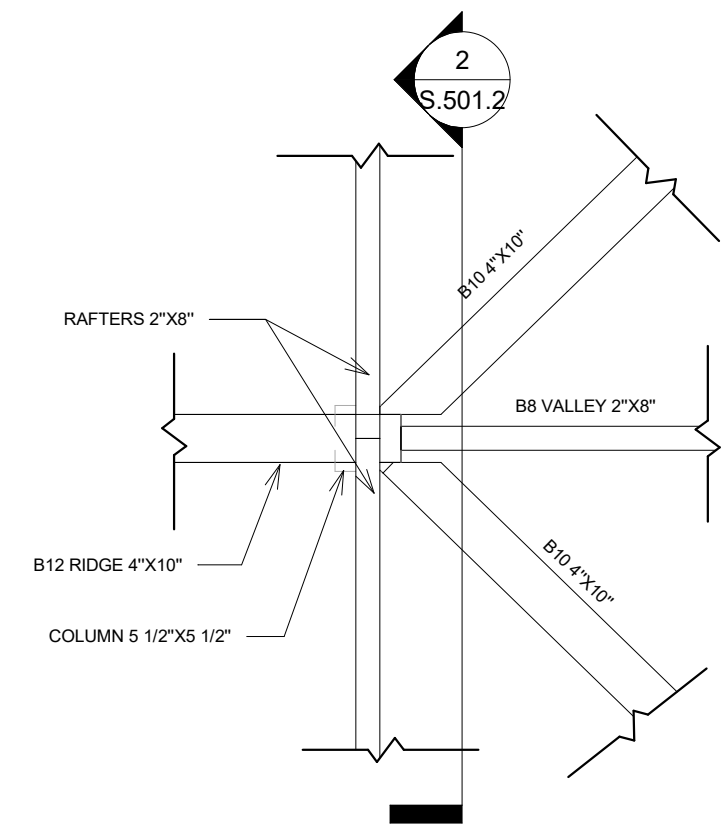
SHEET №



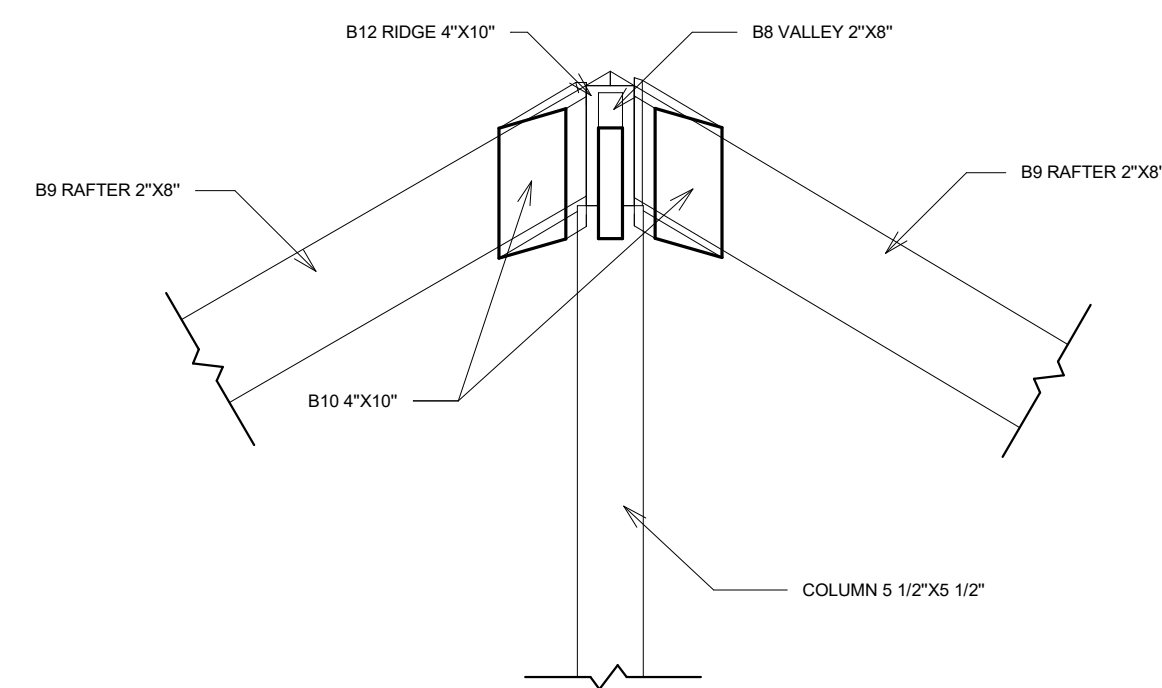
**GDI ENGINEERING**  
INNOVATING SOLUTIONS

GDI Engineering Design  
(Green Dream International LLC)  
10777 Westheimer Rd, Suite 1150  
Houston, TX 77042

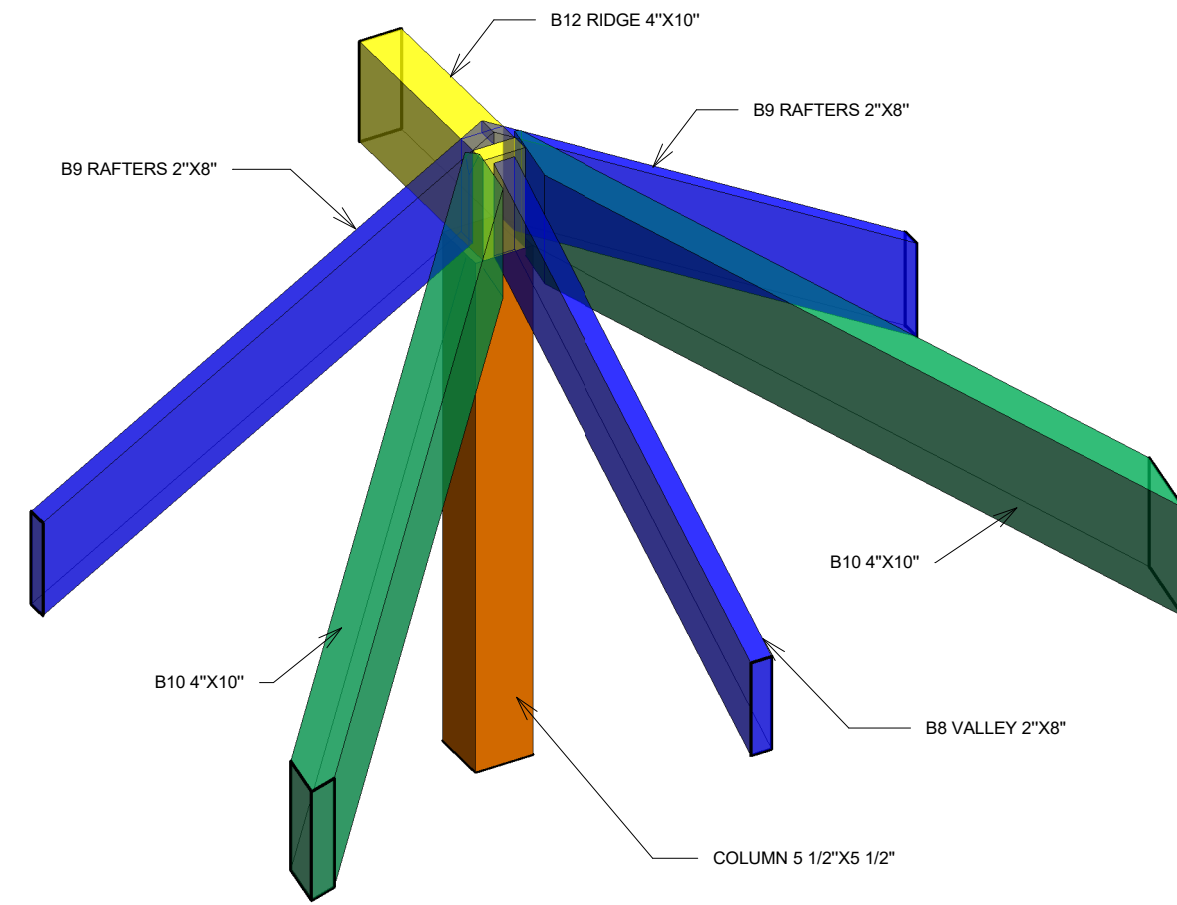
**T2T - DUSTIN WEBER**



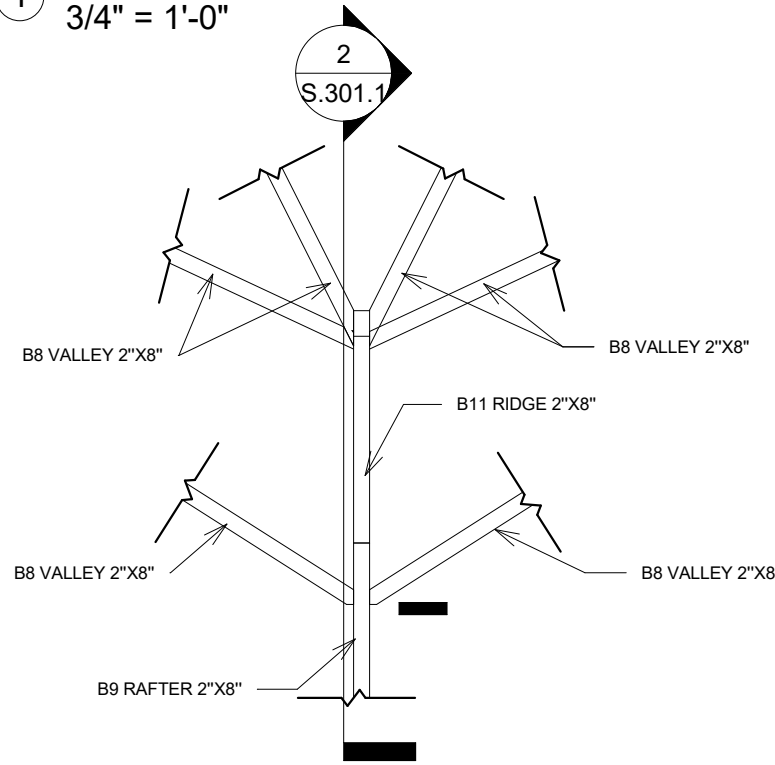
1 POINT 3 TOP  
3/4" = 1'-0"



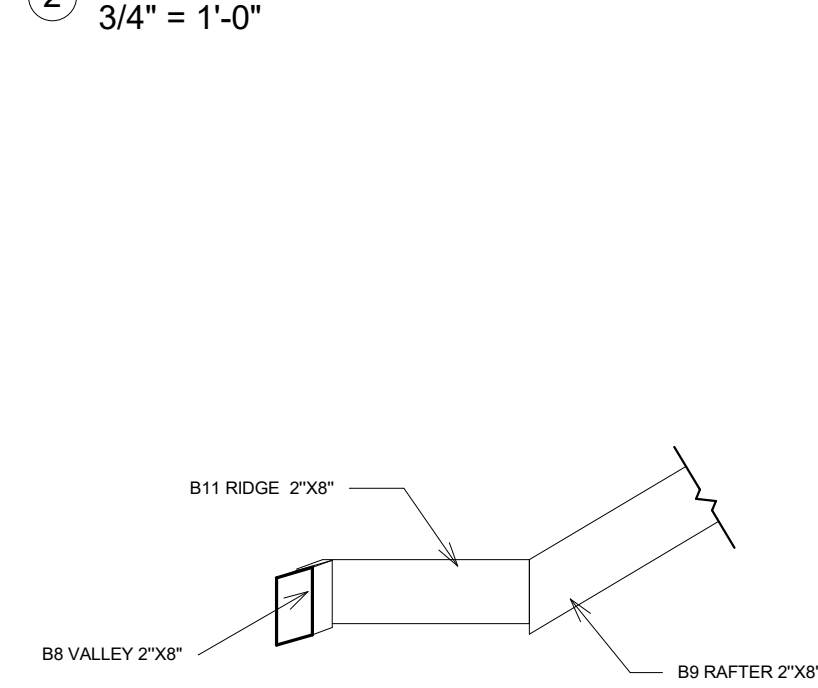
2 POINT 3 FRONT  
3/4" = 1'-0"



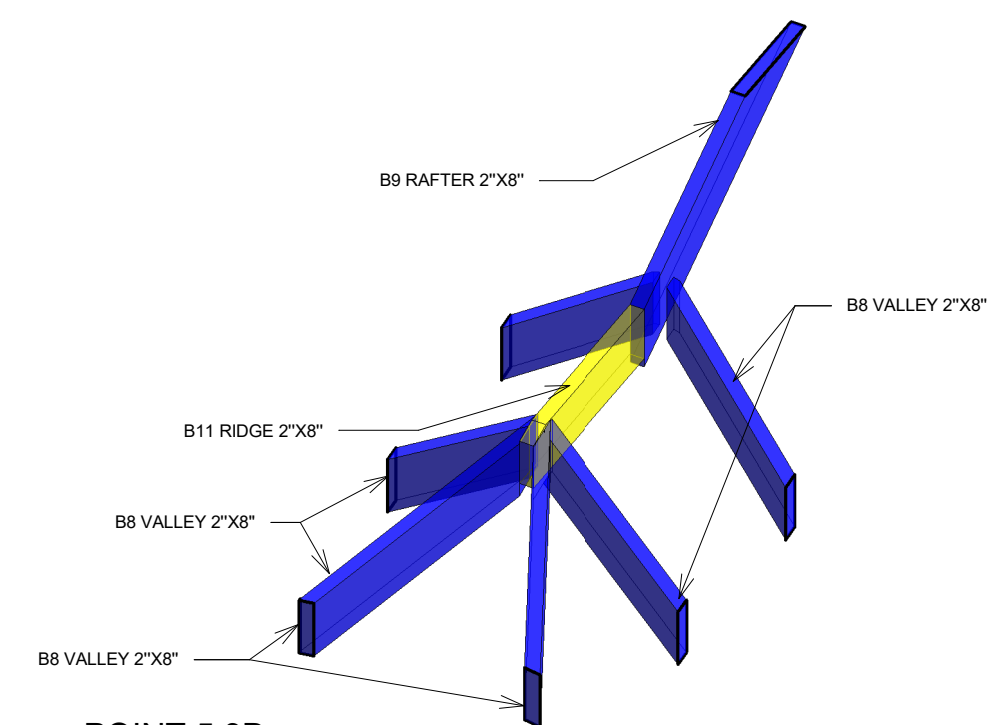
3 POINT 3 3D



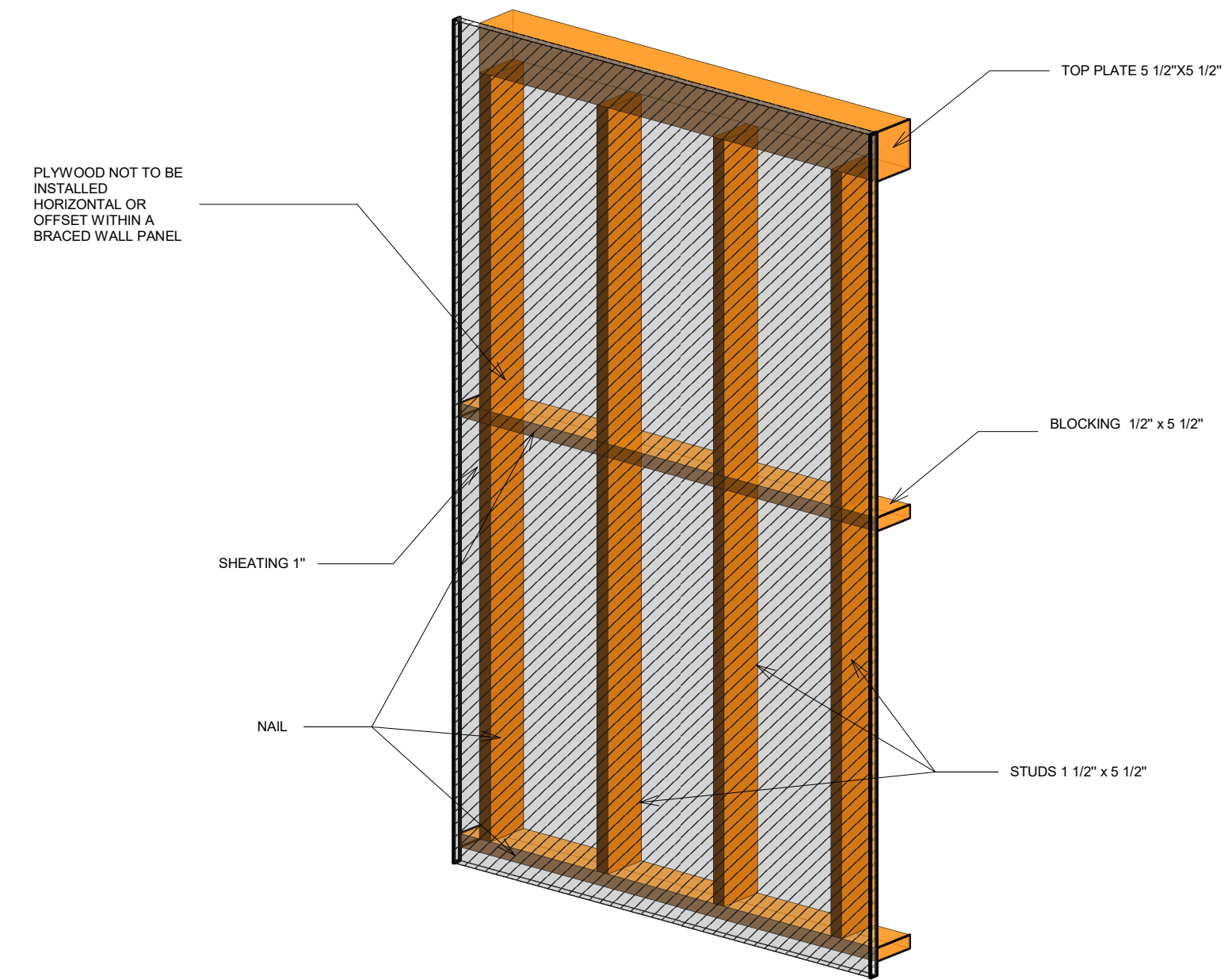
4 POINT 5 TOP  
1/2" = 1'-0"



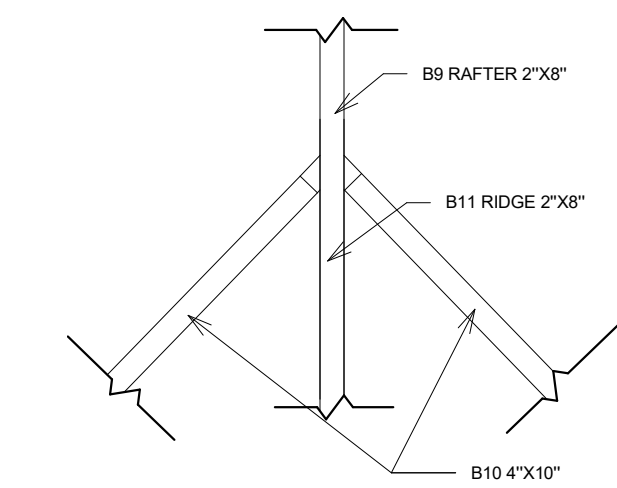
5 POINT 5 FRONT  
1/2" = 1'-0"



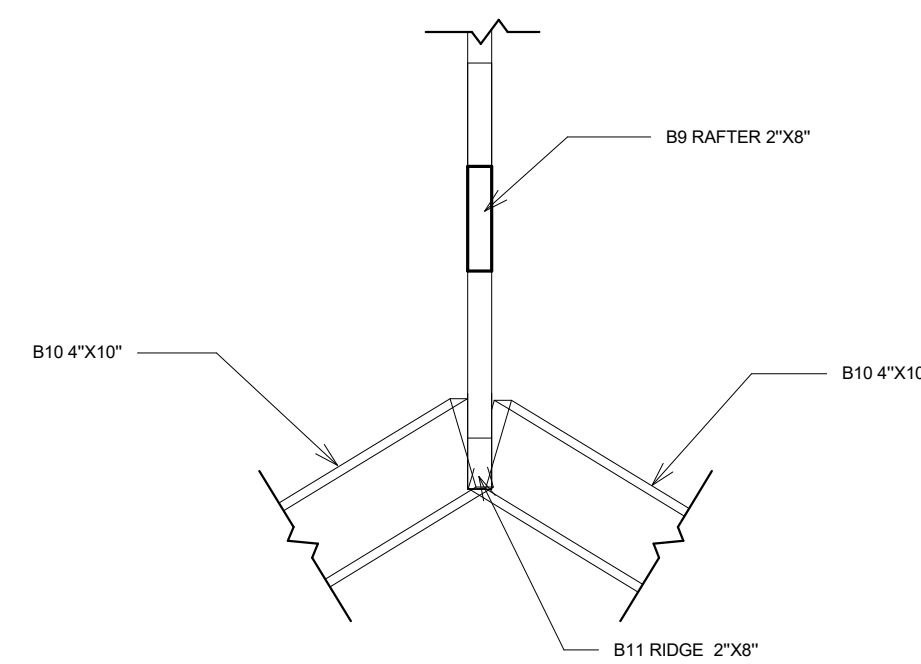
6 POINT 5 3D



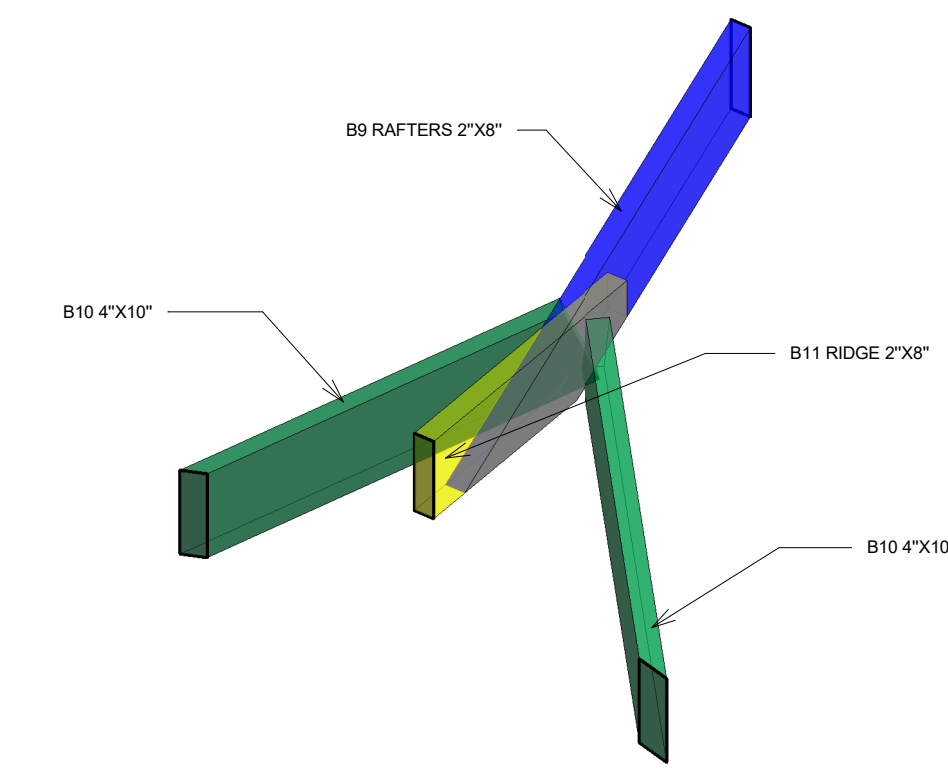
13 TYPICAL FRAMING 3D



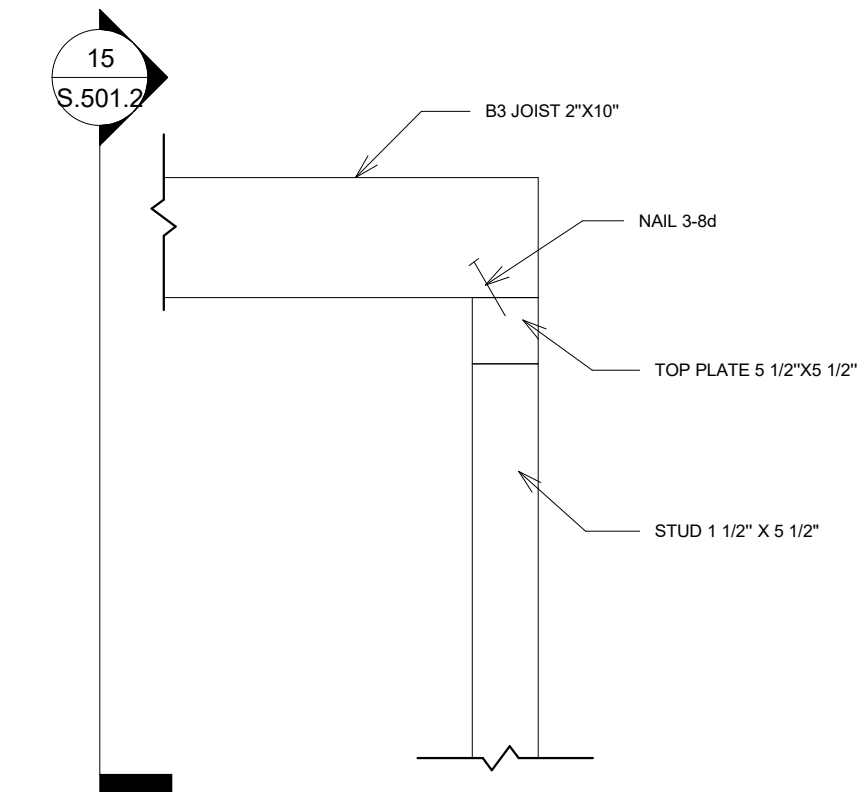
7 POINT 4 TOP  
3/4" = 1'-0"



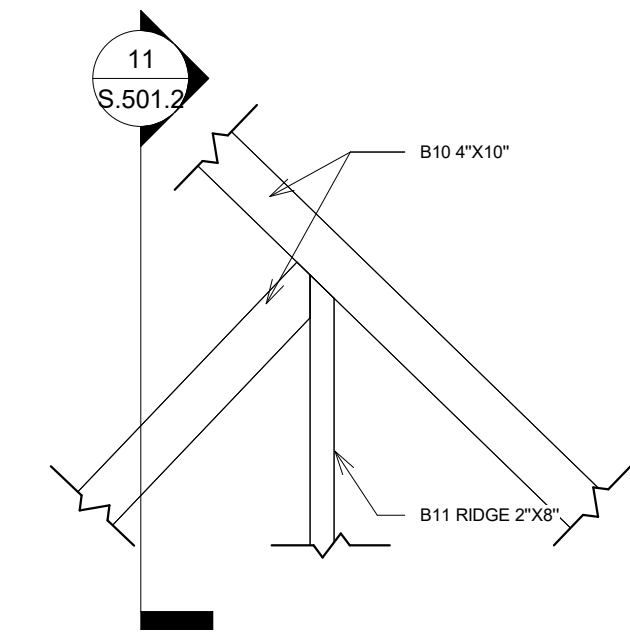
8 POINT 4 LEFT  
3/4" = 1'-0"



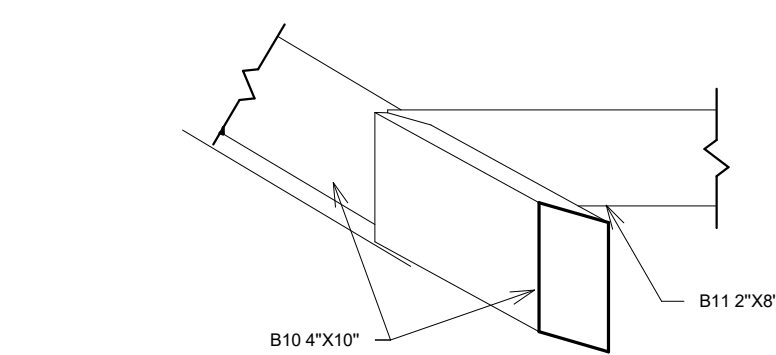
9 POINT 4 3D



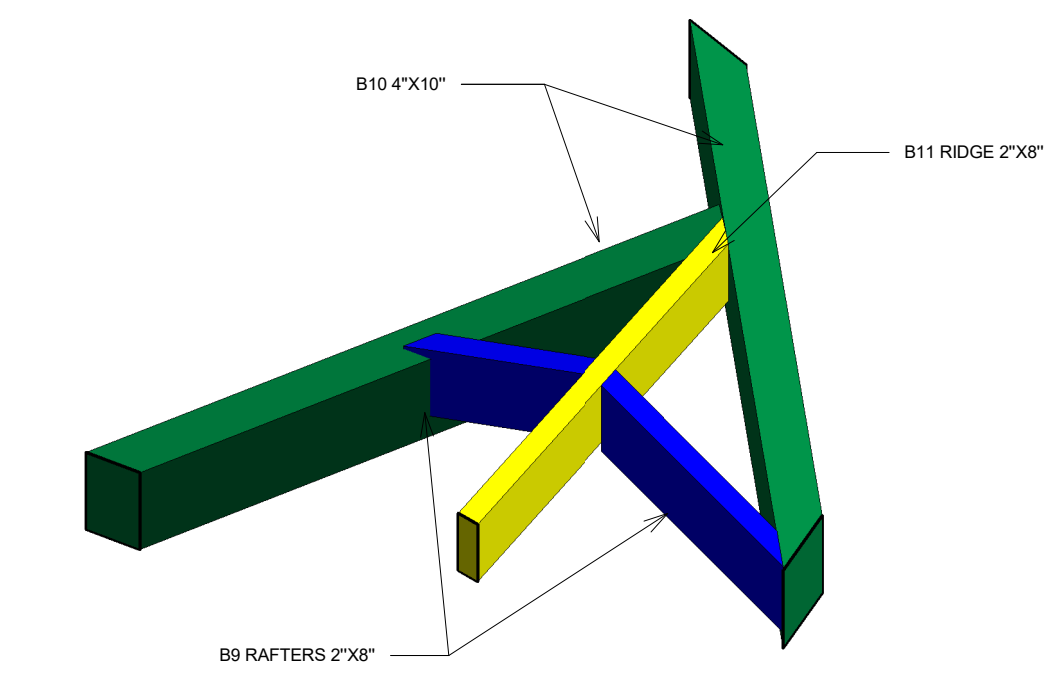
14 CONN JOISTS TO FRAMING FRONT  
3/4" = 1'-0"



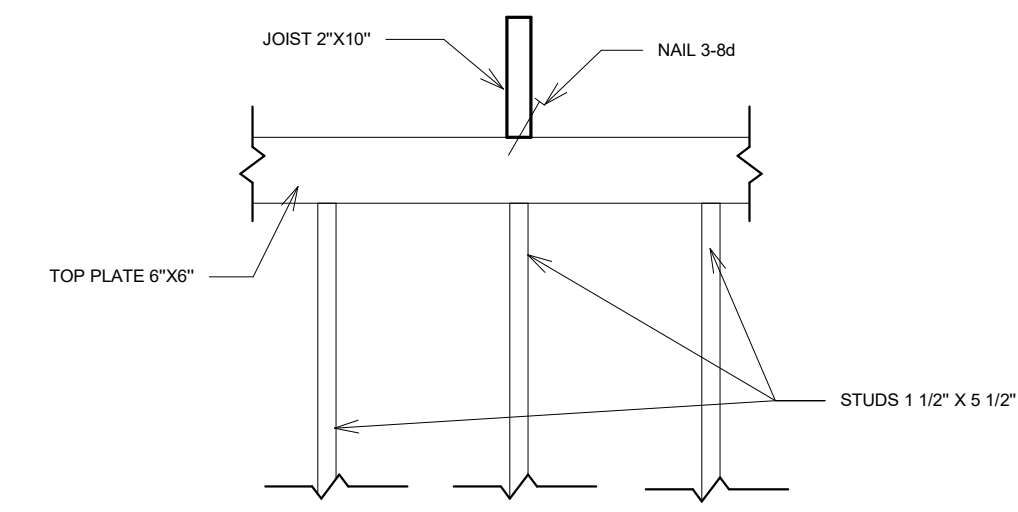
10 POINT 99 TOP  
3/4" = 1'-0"



11 POINT 99 FRONT  
3/4" = 1'-0"



12 POINT 99 3D



15 CONN JOISTS TO FRAMING LEFT  
3/4" = 1'-0"

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**TYPICAL  
CONSTRUCTION  
DETAILS**

Author: \_\_\_\_\_  
Date: \_\_\_\_\_ Project Number: \_\_\_\_\_

**S.501.2**

SHEET №